

Owner Information

Scope of Project

PROPOSED NEW PRE-ENGINEERED, PRECONSTRUCTED
STEEL SHOP BUILDING, ATTACHED TO AN EXISTING
BUILDING OF THE SAME CONST. TYPE AND USE.

PROPOSED ADDRESSING OF AN EXISTING AGG HOUSING
MOBILE HOME. (E) STATE PERMIT IN PLACE

NEW ELECTRICAL METER AT AGG HOUSING.

EXISTING S.F.D. AND POOL TO REMAIN UNALTERED

EXISTING S.F.D.: 2,763 SQ. FT.

EXISTING S.F.D. ATTACHED GARAGE: 400 SQ. FT.

EXISTING AGG MOBILE HOME: 1,000 SQ. FT. (E) STATE PERMIT IN PLACE

EXISTING METAL BUILDING: 1,600 SQ. FT.

PROPOSED METAL BUILDING: 3,674 SQ. FT.

Notes

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****	21 PAGE EMPIRE STEEL MFG DRAWINGS

A vicinity map showing the location of a 'SITE' (shaded gray area) relative to several roads. The map includes the following labels:

- WILT ROAD**: A road segment to the north of the site.
- SITE**: The shaded gray area representing the location of interest.
- PALA MEWSA DRIVE**: A road segment to the west of the site.
- OLD HIGHWAY 395**: A road segment to the east of the site.
- HWY 76**: A road segment that intersects OLD HIGHWAY 395.
- INTERSTATE 15**: A road segment to the east of OLD HIGHWAY 395, indicated by a vertical line with three parallel lines.

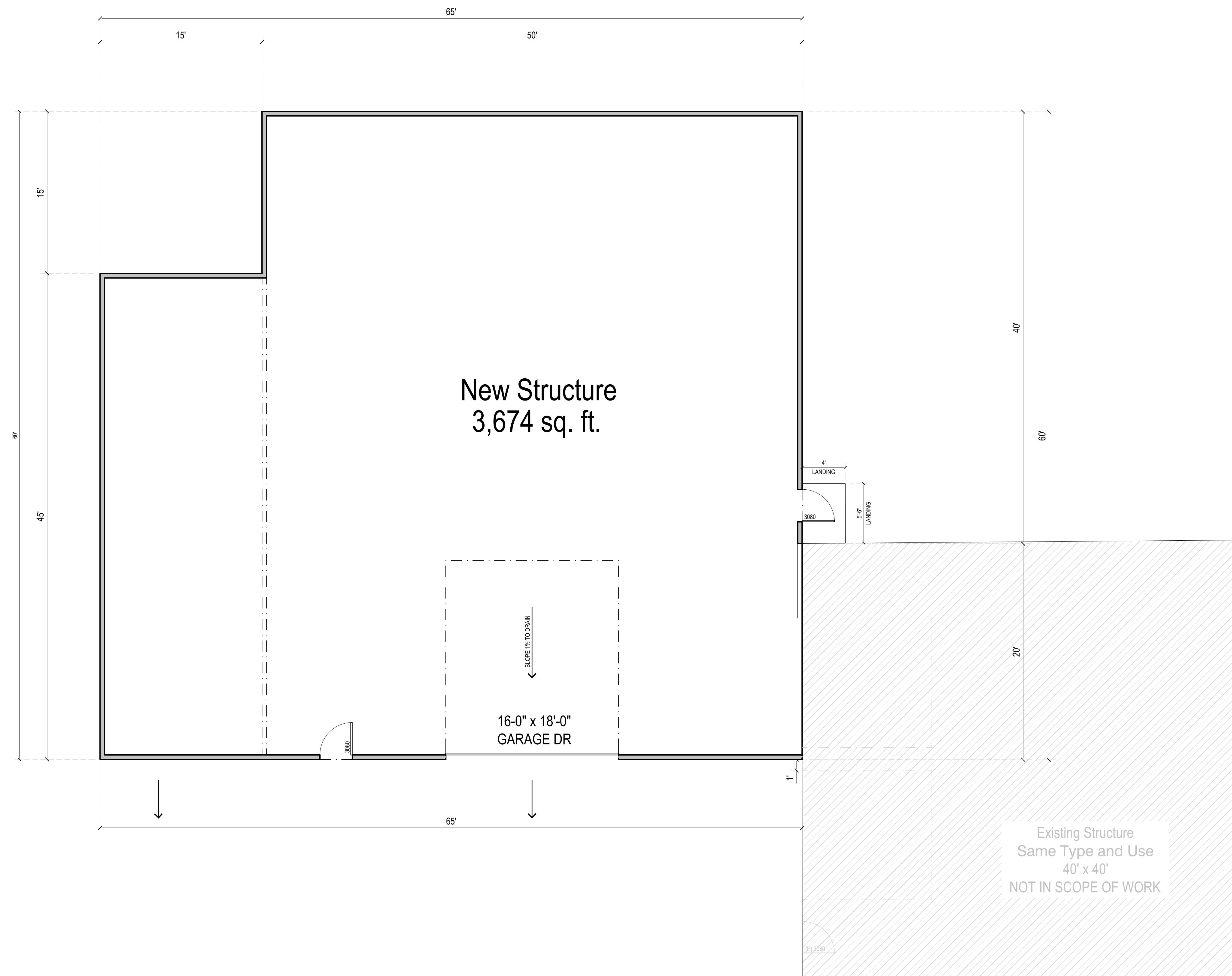
A north arrow is located in the top left corner of the map area.

Brewer Metal Building

2505 Wilt Road
Bur Metal Bu



A. General	
Applicable codes. All projects shall comply with the following building codes and associated County of San Diego amendment, 2022 California Building Code (CBC) and/or California Residential Code (CRC), 2022 California Green Building Standards Code (CalGreen), 2022 California Mechanical Code (CMC), 2022 California Plumbing Code (CPC), 2022 California Fire Code (CFC), 2022 California Building Energy Efficiency Standards (CBEES).	
B. Electrical, Plumbing, and Mechanical	
1. Exterior lighting. All projects shall comply with the County of San Diego lighting ordinance.	
2. GFCI outlets. Ground Fault Circuit Interrupter (GFCI) outlets are required in bathrooms, at kitchen countertops, at laundry and wet bar sinks, in garages, in crawlspaces, in unfinished basements, and outdoors. (CEC 210.8)	
3. AFCI outlets. Electrical circuits in bedrooms, living rooms, dining rooms, dens, closets, hallways, or similar rooms must be protected by Arc Fault Circuit Interrupters (AFCI). (CEC 210.12)	
4. Luminaire requirements. Installed luminaires shall meet the efficacy and fixture requirements of CBEES 150.0(k).	
5. Smoke detectors in building remodels. Smoke detectors are required in each existing sleeping room, outside each separate sleeping area in the immediate vicinity of sleeping rooms, and on the floor of a dwelling including basements. Battery-operated detectors are acceptable in existing areas with no construction taking place and in alterations not resulting in removal of interior wall or ceiling finishes and without access via an atrium, crawl space, or basement. (CRC 314.3)	
6. Carbon monoxide detectors in building remodels. Carbon monoxide detectors are required outside each separate sleeping area in the immediate vicinity of sleeping rooms and on each story of a dwelling including basements. Battery-operated detectors are acceptable in existing areas with no construction taking place and in alterations not resulting in removal of interior wall or ceiling finishes and without access via an atrium, crawl space, or basement. (CRC 315.3)	
7. Water heater seismic strapping. Minimum two 3/4-inch-by-24-inch straps required around water heaters, with 1/4-inch-by-2-inch lag bolts attached directly to framing. Straps shall be at points within upper third and lower third of water heater vertical dimension. Lower connection shall occur minimum 4 inches above controls. (CPC 507.2)	
8. Gas appliances in garages. Water heaters and heating/cooling equipment capable of igniting flammable vapors shall be placed on minimum 18-inch-high platform unless listing report number provided showing ignition-resistant appliance. (CPC 507.13 and CPC 305.1)	
9. Impact protection of appliances. Water heated and heating/cooling equipment subject to potential impact shall be protected by boulders or an equivalent measure. (CPC 507.13.1 and CPC 305.11)	
10. Water closet clearance. Minimum 30-inch-wide by 24-inch-deep clearance required at front of each water closet. (CPC 602.10.2)	
11. Shower valve. Shower components shall have minimum area of 1024 square inches and be able to encompass a 30-inch-diameter circle. Shower doors shall have a minimum 22-inch unobstructed width. (CPC 405.8 and CPC 408.6)	
12. Fireplace appliances. Fireplaces with gas appliances are required to have the flue damper permanently fixed in the open position and fireplaces with LPG appliances are to have no 'pit' or 'sump' configurations. (CPC 307.1)	
13. Chimney clearance. Minimum 2-foot chimney clearance required above building within 10-foot horizontally of chimney. The chimney shall extend minimum 3 feet above highest point where chimney passes through roof. (CRC 1103.9)	
C. Mechanical Ventilation and Indoor Air Quality (ASHRAE 62.2-2010)	
1. Transfer air. Ventilation air shall be provided directly from the outdoors and not as transfer air from adjacent dwelling units or other spaces, such as garages, unconditioned crawlspaces, or unconditioned attics. (CBEES 150.0(i))	
2. Instructions and labeling. Ventilation system controls shall be labeled, and the homeowner shall be provided with instructions on how to operate the system. (CBEES 150.0(i))	
3. Combustion and solid-fuel burning appliances. Combustion appliances shall be properly vented and air systems shall be designed to prevent back drafting. (CBEES 150.0(i))	
4. Garages. The wall and openings between occupiable spaces and the garage shall be sealed. HVAC systems shall include air handlers or ducts located in garages shall have total air flow of no less than 60% of total air flow when measured at 0.1 in. w.c. using California Title 24 or equivalents. (CBEES 150.0(i))	
5. Minimum filtration. Mechanical systems supplying air to occupiable space through ductwork shall be provided with a filter having a minimum efficiency of MERV 6 or better. (CBEES 150.0(i))	
6. Air inlets. Air inlets (exhaust) shall be located away from known contaminants. (CBEES 150.0(i))	
7. Air moving equipment. Air moving equipment used to meet either the whole-building ventilation requirement or the local ventilation exhaust requirement shall be rated in terms of airflow and sound. (CBEES 150.0(i))	
a. All continuously operating fans shall be rated at a maximum of 1.0 sone.	
b. Intermittently operated whole-building ventilation fans shall be rated at a maximum of 1.0 sone.	
c. Intermittently operated local exhaust fans shall be rated at a maximum of 3.0 sone.	
d. Remotely located air-moving equipment (mounted outside of habitable spaces) need not meet sound requirements if at least 4 feet of ductwork between fan and intake grill.	
D. Foundation and Underfloor	
1. Foundation reinforcement. Continuous footings and stem walls shall be provided with a foundation for two longitudinal 4 No. 4 bars, one at the top and one at the bottom of the footing. (CRC R403.1.3)	
2. Shear wall foundation support. Shear walls shall be supported by continuous foundations. (CRC 403.1.2)	
3. Concrete slabs-on-grade. Slabs-on-grade shall be minimum 3-1/2-inches thick. (CRC R506.1)	
4. Water retention. A 10-mil polyethylene or approved vapor retarder with joints lapped minimum 6 inches shall be placed between a concrete slab-on-grade and the base course or subgrade. (CRC 506.2.3)	
5. Anchor bolts and stiles. Foundation plates or stiles shall be bolted or anchored to the foundation or foundation wall per the following (CRC R403.1.6 and CRC R602.11.1):	
a. Minimum 1/2-inch-diameter steel bolts	
b. Bolts embedded at least 7 inches into concrete or masonry	
c. Bolts spaced maximum 6 feet on center	
d. Minimum two bolts per plate/size with one bolt located maximum 12 inches and minimum 7 inches from the center of each size of each plate/piece	
e. Minimum 3-inch by 299-inch steel plate washer between sill and nut on each bolt	
6. Hold-downs. All hold-downs must be tied in place prior to foundation inspection.	
7. Protection of wood against decay. Naturally durable or preservative-treated wood shall be provided in the following locations (CRC 317.1):	
a. All wood in contact with ground, embedded in concrete in direct contact with ground, or embedded in concrete exposed to weather	
b. Wood joints within 18 inches and wood fibers within 12 inches of the exposed ground in crawl spaces shall be of naturally durable or preservative-treated wood	
c. Wood framing members that rest on concrete or masonry exterior foundation walls and are less than 8 inches from exposed earth shall be of naturally durable or preservative-treated wood	
d. Wood framing, sheathing, and siding on the exterior of the building and having clearance less than 6 inches from the exposed ground or less than 2 inches vertically from concrete steps, porch slab, stairs, and similar horizontal surface exposed to weather	
e. Sills, sleepers or concrete or masonry slab in direct contact with ground unless separated from such slab by impermeable moisture barrier	
f. Ends of wood girders entering masonry or concrete walls with clearances less than 1/2 inch on tops, sides, and ends	
g. Wood structural members supporting moisture-permeable floors or roofs exposed to weather, moisture, or water damage	
h. Wood framing strips or other wood framing members attached directly to interior of exterior concrete or masonry walls before grade except where vapor retarder applied between wall and framing strips or framing members	
8. Underfloor ventilation. Underfloor areas shall have ventilation openings through foundation walls or exterior walls, with minimum net area of ventilation openings of 1 square foot for each 150 square feet of underfloor area. On such venting/venting opening shall be within 3 feet of each corner of the building. (CRC R408.1)	
9. Underfloor access. Underfloor areas shall be provided with a minimum 18-inch by 24-inch access opening. (CRC R408.4)	
E. Wood Framing	
1. Fastener requirements. The number, size, and spacing of fasteners connecting wood members/elements shall not be less than that set forth in CRC Table R602.3(1). (CRC R502.9, CRC R602.3, and CRC R602.2)	
2. Stud size, height, and spacing. The size, height, and spacing of studs shall be in accordance with CRC Table R602.3(5). (CRC R602.3)	
E. Wood Framing (Continued)	
3. Sill plate. Studs shall have full bearing on nominal 2-inch thick or larger sill plate with width at least equal to stud width. (CRC R602.3.4)	
4. Bearing studs. Where joists, trusses, or beams are spaced more than 16 inches on center and the joist span exceeds 24 inches on center, such members shall bear within 5 inches of the stud beneath. (CRC R602.3.3)	
5. Drilling and notching of studs. Any stud in an exterior wall or bearing partition may be cut or notched to a depth not exceeding 25% of its width. Studs in nonbearing partitions may be notched to a depth not to exceed 40% of a single stud width. Any stud may be bored or drilled, provided the diameter of the resulting hole is no more than 60% of the stud width, the edge of the hole is no more than 5/8 inch to the edge of the stud, and the hole is not located in the same area as a cut or notch. Studs located in exterior wall or bearing partitions drilled over 40% and up to 60% shall also be doubled with no more than two successive studs bored. (CRC R602.6)	
6. Top plates. Wood stud walls shall be capped with a double top plate installed to provide overlapping at corners and at intersections with other partitions. End joints in double top plates shall be offset at least 24 inches. Joints in plates need not occur over studs. Plates shall be minimum nominal 2 inches thick and have width at least equal to width of studs. (CRC R602.3.2)	
7. Top plate splices. Top plate lap splices shall be connected to top plates of adjacent wall sections with blocking between the top plates. (CRC R602.10.8)	
8. Roofing and weatherproofing (Continued)	
38. Framing of roofing. Openings. Openings in roof and ceiling framing shall be framed with a single header timber. When the header joist span does not exceed 4 feet, the header joist may be a single member the same size as the ceiling joist or rafter. Single joist header timbers may be used to carry a single header joist located within 3 feet of the trimmer joist bearing. When the header joist span exceeds 4 feet, the trimmer joist and header joist shall be doubled and of sufficient cross section to support the ceiling joists or rafters framing into the header. Approved penetrations and building appendages in a manner to maintain a weather-resistant exterior wall envelope. (CRC R703.2)	
39. Roof framing above shear walls. Rafters or roof trusses shall be connected to top plates of shear walls with blocking between the rafters or trusses. (CRC R602.10.8)	
40. Roof diaphragm under fill framing. Roof plywood shall be continuous under California fill framing.	
41. Roof diaphragm at ridges. Minimum 2-inch nominal blocking required for roof diaphragm nailing at ridges.	
42. Blocking of roof trusses. Minimum 2-inch nominal blocking required between trusses at ridge lines and at points of bearing at exterior walls.	
43. Truss clearance. Minimum 1/2-inch clearance required between top plates of interior non-bearing partitions and bottom chords of trusses.	
44. Drilling, cutting, and notching of roof/floor framing. Notches in solid lumber joists, rafters, blocking, and beams shall not exceed one-sixth the member depth, shall not be greater than one-third the member depth, and shall not be located in the middle third of the span. Notches in member ends shall be cut outwards from the member ends. The total width of notches 4 inches or greater in nominal thickness shall not be notched except at member ends. The diameter of holes bored or cut into member shall not exceed one-third the member depth. Holes shall not be closer than 2 inches to the top or bottom of the member or to any other hole located in the member. Where the member is also notched, the hole shall not be closer than 2 inches to the top or bottom of the member. (CPC R502.8)	
45. Exterior framing. Exterior framing, including batten, blocking, and trim shall be self-supporting. Attachment to the structure shall be accomplished by use of toenails or nails subject to withdrawal. (CRC R311.3)	
46. Wall bracing. Buildings shall be braced in accordance with the methods allowed per CRC R602.10, CRC R602.10.4, and/or CRC R602.10.5.	
47. Brace wall line spacing. Spacing between brace wall lines shall not exceed 20 feet or exceed provisions of CRC R602.10.13.	
48. Shear wall cumulative length. The cumulative length of shear walls within each braced wall line shall meet the provisions of CRC Table R602.10.3(1) for wind loads and CRC Table R602.10.3(2) for seismic loads.	
49. Shear wall spacing. Shear walls shall be located no more than 25 feet on center. (CRC R602.10.4)	
50. Shear wall offset. Shear walls may be offset out-of-plane not more than 4 feet from the designed brace wall line and not more than 8 feet from any other offset wall considered part of the same braced wall line. (CRC R602.10.12)	
51. Shear wall location. Shear walls shall be located at the ends of each braced wall line or meet the alternate provisions of CRC R602.10.2.2.	
52. Individual shear wall length. Shear walls shall meet minimum length requirements of CRC R602.10.6.5.	
53. Cripple wall bracing. Cripple walls shall be braced by CRC R602.10.11.	
54. Shear wall and diaphragm nailing. All shear walls, roof diaphragms, and floor diaphragms shall be nailed to supporting construction per CRC Table R602.3(1). (CRC R604.3)	
55. Shear wall joints. All vertical joints in shear wall sheathing shall occur over, and be fastened to, common studs. Horizontal joints in shear walls shall occur over, and be fastened to, minimum 1-1/2-inch-thick blocking. (CRC R602.10.10)	
56. Shear wall openings. Headers, double joists, or trusses of adequate size to transfer loads to vertical members shall be provided over window and door openings in load-bearing walls and partitions. (CBC 2304.3.2)	
57. Joists under bearing partitions. Joists under parallel bearing partitions shall be of adequate size to support the load, and the size of each joist shall be determined by the joist span and the joist depth. Joists shall be self-supporting or placed on studs or staggered studs. Unfaced fiberglass bat insulation used for fireblocking shall fit the entire cross-section of the shear wall to a minimum height of 16 inches. Insulation shall be applied vertically in pipe, corner, or similar configurations. (CPC 4.40.3)	
58. Joists above or below shear wall. Where joists are perpendicular to a shear wall above or below, a rim joist band, or blocking shall be provided along the entire length of the shear wall. Where joists are parallel to a shear wall above or below, a rim joist, end joist, or parallel joist band shall be provided across the entire width of the shear wall. Joists shall be self-supporting or offset from supporting girders, walls, or partitions more than 1/2 inch. (CPC 4.40.3)	
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PROPOSED FLOOR PLAN

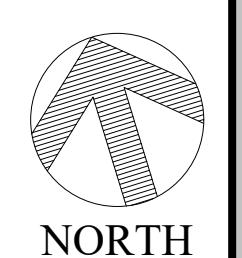
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Wall Legend

— New Steel Walls

Floor plan:

3,674 sq. ft.



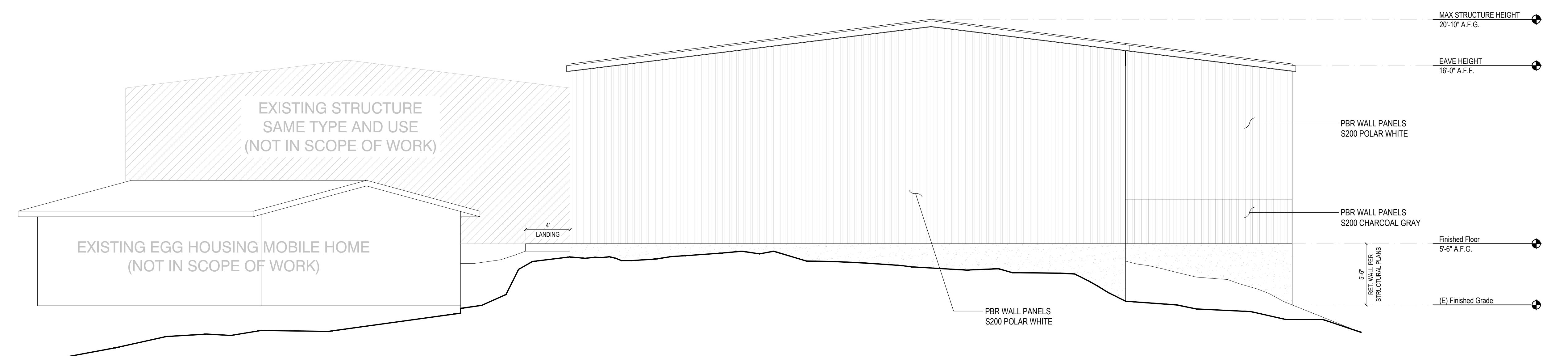
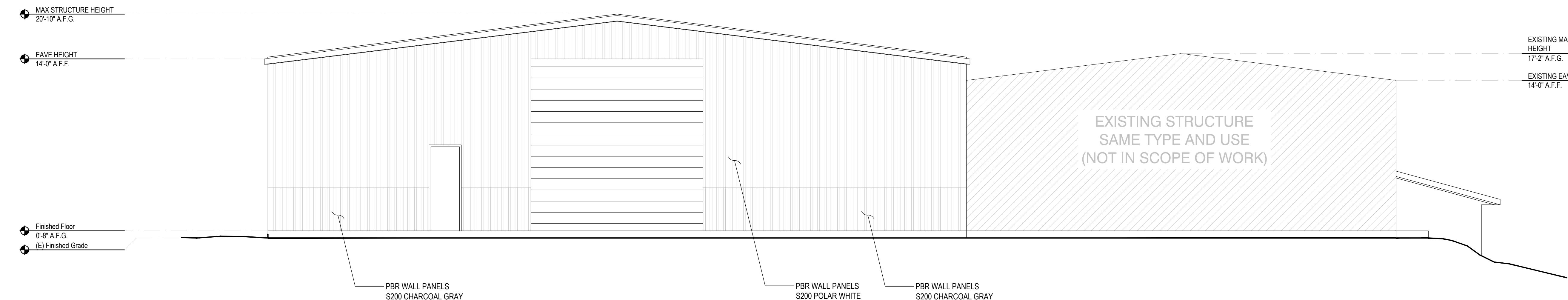
A-1

Sheet:

Date: 12/15/25
Scale: 3/16" = 1'-0"

Sheet Title: Proposed Floor Plan
Plan Prepared by: Fernando Martinez
Signature

MADDESIGN
& DRAFTING SERVICES
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Brewer Detached Structure

2505 Wilt Rd.
Fallbrook, CA 92028

Elevations

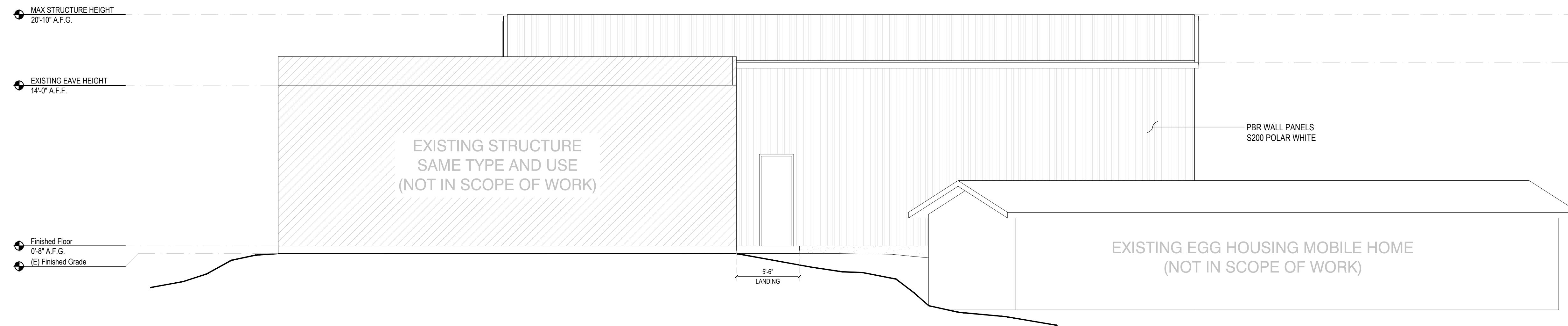
Plan Prepared by: Fernando Martinez
Fernando Martinez

Sheet Title:
Elevations

Rev. Date By
12/15/25

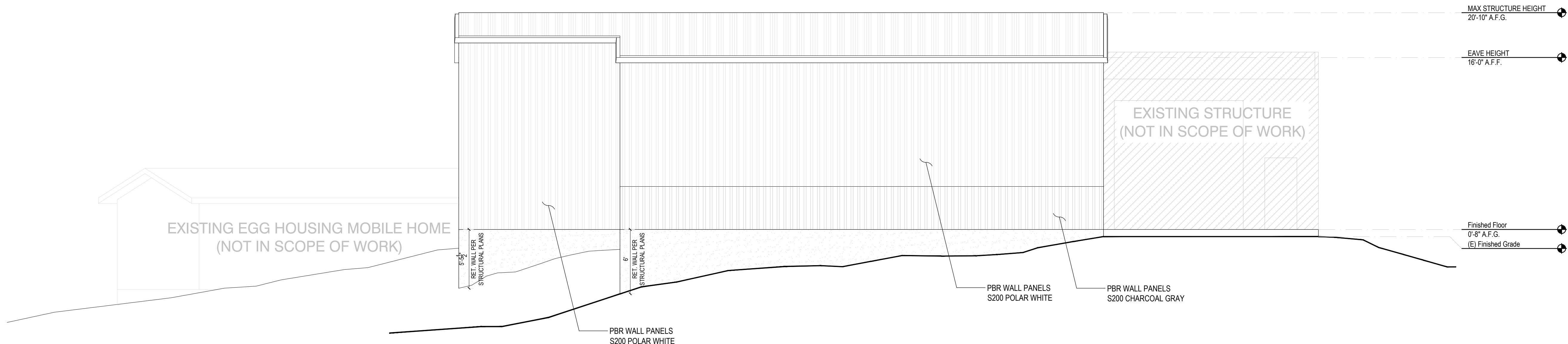
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A-2



East Elevation

Scale: $\frac{3}{16}$ " = 1'-0"



West Elevation

Scale: $\frac{3}{16}$ " = 1'-0"

Brewer Detached Structure

2505 Wilt Rd.
Fallbrook, CA 92028

Elevations

Sheet Title:

Fernando Martinez

[Signature]

Plan Prepared by:

Fernando Martinez

[Signature]

Date:

12/15/25

Scale:

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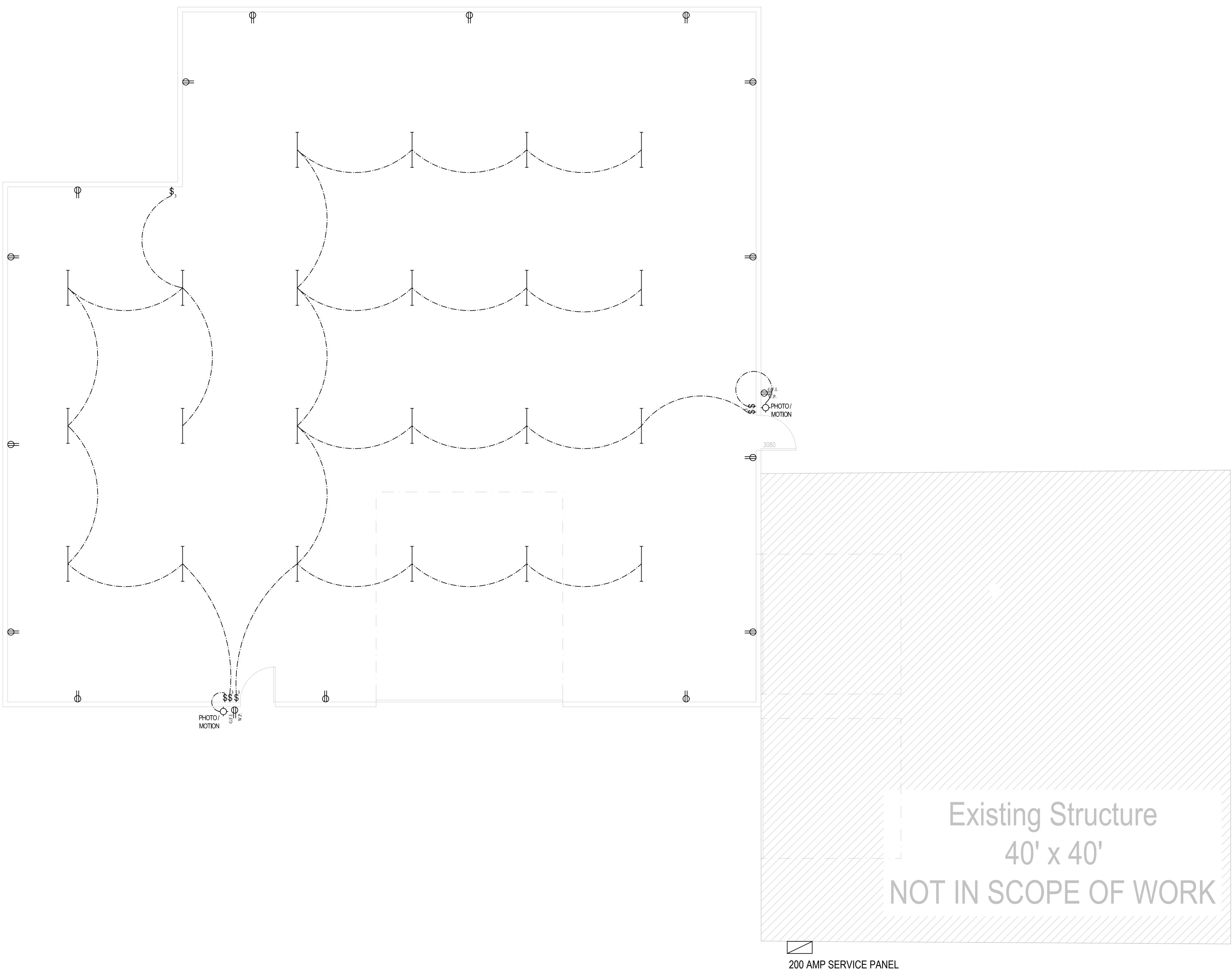
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A-3

Rev.

Date

By



ELECTRICAL PLAN

ELECTRICAL PLAN NOTES:

- ELECTRICAL SYSTEM GROUND TO BE PROVIDED PER NEC ARTICLE 250-80(s), 81(c).
- ELECTRICAL OUTLETS SHALL BE INSTALLED IN ACCORDANCE WITH NEC ARTICLE 210-52(a).
- LIGHT FIXTURES INSTALLED IN CLOTHES CLOSETS SHALL COMPLY WITH NEC ARTICLE 410-8.
- LIGHTING OUTLETS CONTROLLED BY A SWITCH SHALL BE PROVIDED IN ACCORDANCE WITH NEC ARTICLE 210-8.
- BATHROOM RECEPTACLE OUTLETS SHALL BE SUPPLIED BY A MINIMUM OF ONE 20 AMPERE BRANCH CIRCUIT. SUCH CIRCUITS SHALL HAVE NO OTHER DEVICES. THIS CIRCUIT MAY SERVE MORE THAN ONE BATHROOM OR, EACH BATHROOM IS TO BE ON ITS OWN DEDICATED 20 AMPERE CIRCUIT WITH ONLY THAT BATHROOM'S DEVICES ON THAT CIRCUIT.
- CONVENIENCE OUTLETS IN BATHROOMS, KITCHENS COUNTERTOPS WITHIN 6' OF THE OUTDOORS, GARAGES, AND BASEMENTS (OTHER THAN FOR LAUNDRY OR SIMILAR EQUIPMENT) SHALL BE G.F.C.I. PROTECTED. ALL KITCHEN COUNTERTOP OUTLETS SHALL BE G.F.C.I. PROTECTED. NEC ARTICLE 210-8.
- BEDROOM BRANCH CIRCUITS TO BE ARC FAULT CIRCUIT PROTECTED.
- ALL SMOKE DETECTORS ARE TO BE PERMANENTLY WIRED WHICH WILL SOUND AN ALARM WHEN ACTUATED. SEE LEGEND, ALARM IS TO BE AUDIBLE IN ALL SLEEPING AREAS OF THE UNIT. SMOKE DETECTORS ARE REQUIRED TO BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTIVATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS. IN NEW CONSTRUCTION SMOKE ALARMS SHALL RECEIVE THEIR PRIMARY SOURCE FROM THE BUILDING WIRING AND SHALL BE EQUIPPED WITH BATTERY BACKUPS AND LOW BATTERY SIGNAL.
- THE INSTALLATION OF SMOKE ALARMS AND SMOKE DETECTORS SHALL COMPLY WITH THE SPECIFIC LOCATION REQUIREMENTS OF CRC R314.3.4.
- SMOKE ALARMS AND SMOKE DETECTORS SHALL BE INSTALLED A MINIMUM 20 FEET HORIZONTAL DISTANCE FROM A PERMANENTLY INSTALLED COOKING APPLIANCE.
- SMOKE ALARMS SHALL BE INSTALLED NOT LESS THAN A 3-FOOT HORIZONTAL DISTANCE FROM THE DOOR OR OPENING OF A BATHROOM THAT CONTAINS A BATHTUB OR SHOWER UNLESS THIS WOULD PREVENT PLACEMENT OF A SMOKE ALARM REQUIRED BY OTHER SECTIONS OF THE CRC.
- SMOKE ALARMS AND SMOKE DETECTORS SHALL NOT BE INSTALLED WITHIN A 36-INCH HORIZONTAL PATH FROM THE SUPPLY REGISTERS OF A FORCED AIR HEATING OR COOKING SYSTEM AND SHALL BE INSTALLED OUTSIDE OF THE DIRECT AIRFLOW OF THOSE REGISTERS.
- SMOKE ALARMS SHALL COMPLY WITH NFPA 72 AND SHALL BE LISTED IN ACCORDANCE WITH UL 217.
- COMBINATION SMOKE AND CARBON MONOXIDE ALARMS SHALL BE LISTED IN ACCORDANCE WITH UL 217 AND UL 2034.
- SMOKE ALARM SYSTEMS AND COMPONENTS SHALL BE CALIFORNIA STATE FIRE MARSHALL LISTED AND APPROVED IN ACCORDANCE WITH CALIFORNIA CODE OF REGULATIONS, TITLE 19, DIVISION 1 FOR THE PURPOSE FOR WHICH THEY ARE INSTALLED.
- FOR WATER HEATER AND F.A.U. (IF OCCURS) MINIMUM SPECIFICATIONS, SEE TITLE-24 AND ENERGY SPECIFICATIONS OF THIS SET.
- WATER HEATER AND F.A.U. (IF FLOOR MOUNT OCCURS) TO BE ON 18" HIGH PLATFORM.
- LIGHTING IN BATHROOMS SHALL HAVE ALL HIGH EFFICACY LUMINARIES AND AT LEAST ONE LUMINARIES MUST BE CONTROLLED BY A VACANCY SENSOR.
- OTHER ROOMS: ALL LUMINARIES SHALL BE HIGH-EFFICACY AND HAVE A MANUAL ON/OFF IN ADDITION TO A VACANCY SENSOR OR DIMMER.
- KITCHENS: ALL THE INSTALLED WATTAGE OF LUMINARIES IN KITCHENS SHALL BE HIGH EFFICACY AND SHALL HAVE A MANUAL ON/OFF IN ADDITION TO A VACANCY SENSOR OR DIMMER. UNDER CABINET LIGHTING SHALL BE SWITCHED SEPARATELY.
- OUTDOOR LIGHTING: ALL LUMINARIES MOUNTED TO THE BUILDING OR TO OTHER BUILDINGS ON THE SAME LOT SHALL BE HIGH EFFICACY LUMINARIES AND MUST BE CONTROLLED BY MANUAL ON AND OFF SWITCH, AND USE ONE OF THESE AUTOMATIC CONTROL TYPES:

 - PHOTO CONTROL AND MOTION SENSOR
 - PHOTO CONTROL AND AUTOMATIC TIME SWITCH CONTROL
 - ASTRONOMICAL TIME CLOCK
 - ENERGY MANAGEMENT CONTROL SYSTEM PER CBEES 150.0(k)3aiic.

- GARAGES, LAUNDRY ROOMS AND UTILITY ROOMS: ALL LUMINARIES SHALL BE HIGH EFFICACY AND AT LEAST ONE LIGHTING FIXTURE IN EACH OF THESE SPACES SHALL BE CONTROLLED BY A VACANCY SENSOR.
- RECESSED DOWN-LIGHT LUMINARIES IN CEILINGS SHALL NOT BE SCREW BASED.
- ALL LUMINARIES REQUIRING "JA8-2016" OR "JA8-2016-E" MARKING SHALL BE CONTROLLED BY A DIMMER OR VACANCY SENSOR.
- ALL LED LUMINARIES AND LAMPS SHALL BE MARKED "JA8-2016" AND LISTED IN THE CALIFORNIA ENERGY COMMISSION DATABASE AT <https://CACERTAPPLIANCES.ENERGY.CA.GOV/PAGES/APPLIANCESEARCH.ASPX>
- ALL RECESSED DOWNLIGHT AND ENCLOSED LUMINARIES SHALL BE MARKED "JA8-2016" AND LISTED IN THE CALIFORNIA ENERGY COMMISSION DATABASE AT <https://CACERTAPPLIANCES.ENERGY.CA.GOV/PAGES/APPLIANCESEARCH.ASPX>
- A MECHANICAL EXHAUST SYSTEM, SUPPLY SYSTEM, OR COMBINATION THEREOF SHALL BE INSTALLED FOR EACH DWELLING UNIT TO PROVIDE WHOLE-BUILDING VENTILATION WITH OUTDOOR AIR COMPLYING WITH ASHRAE STANDARD 62.2 AS ADOPTED BY THE CALIFORNIA ENERGY COMMISSION. HERS VERIFICATION REQUIRED TO CONFIRM WHOLE-BUILDING VENT AIRFLOW.
- AN INTERMITTENTLY OR CONTINUOUSLY OPERATING LOCAL MECHANICAL EXHAUST SYSTEM (WITH OUTDOOR AIR) SHALL BE INSTALLED IN EACH KITCHEN AND BATHROOM COMPLYING WITH ASHRAE STANDARD 62.2-2007 AS ADOPTED BY THE CALIFORNIA ENERGY COMMISSION. INTERMITTENT LOCAL VENTILATION EXHAUST AIRFLOW RATES SHALL BE 50 CFM IN BATHROOMS, AND 5 ACH (AIR CHANGES/ HOUR) IN KITCHENS BASED ON KITCHEN VOLUME.
- GAS-FIRED WATER HEATERS AND FURNACES LOCATED IN BEDROOMS OR BATHROOMS SHALL COMPLY WITH ONE OF THE FOLLOWING (CPC 505.1, CMC 904.1): INSTALLED IN DEDICATED CLOSET WITH LISTED, GASKETED, SELF-CLOSING DOOR WITH ALL COMBUSTION AIR FROM THE OUTDOORS, OR WATER HEATER OR FURNACE SHALL BE A DIRECT-VENT APPLIANCE.
- DRYER VENTS SHALL BE PER THE FOLLOWING (CMC 504.3.2):
- MINIMUM 4-INCH DIAMETER, MAXIMUM 14-FOOT COMBINED HORIZONTAL AND VERTICAL LENGTH WITH TWO 90-DEGREE ELBOWS, AND TWO FEET DUCTED FROM MAXIMUM LENGTH FOR EACH ELBOW IN EXCESS OF TWO.
- EXHAUST DUCTS AND DRYER VENTS SHALL BE EQUIPPED WITH BACK-DRAFT DAMPERS.
- ENVIRONMENTAL AIR DUCTS AND EXHAUST TERMINATIONS SHALL TERMINATE NOT LESS THAN 3 FEET FROM A PROPERTY LINE AND 3 FEET FROM OPENINGS INTO THE BUILDING.
- ATTIC INSTALLATION MUST COMPLY WITH SECTIONS 904, 908, AND 909 OF THE CALIFORNIA MECHANICAL CODE (CMC).
- A LISTED RACEWAY SHALL BE PROVIDED TO FACILITATE FUTURE INSTALLATION OF ELECTRIC VEHICLE CHARGER IN NEW ONE- AND TWO-FAMILY DWELLINGS AND TOWNHOUSES WITH ATTACHED PRIVATE GARAGES.
- RACEWAY SHALL NOT BE LESS THAN TRAD SIZE 1 (NOMINAL 1-IN. INSIDE DIAMETER) TO ACCOMMODATE A DEDICATED 208/240-VOLT BRANCH CIRCUIT.
- THE EVCS RACEWAY SHALL ORIGINATE AT THE MAIN SERVICE OR SUBPANEL AND TERMINATE INTO A LISTED CABINET, BOX OR OTHER ENCLOSURE IN CLOSE PROXIMITY TO THE PROPOSED LOCATION OF THE EV SPACE.
- THE EVCS RACEWAY SHALL BE CONTINUOUS AT ENCLOSED, INACCESSIBLE OR CONCEALED AREAS AND SPACES.
- THE EVCS SERVICE PANEL OR SUBPANEL SHALL PROVIDE CAPACITY TO INSTALL A 40-AMPERE MINIMUM DEDICATED BRANCH CIRCUIT AND SPACE(S) RESERVED TO PERMIT INSTALLATION OF A BRANCH CIRCUIT OVERCURRENT PROTECTIVE DEVICE.
- THE EVCS SERVICE PANEL OR SUBPANEL CIRCUIT DIRECTORY SHALL IDENTIFY:

 - THE OVERCURRENT PROTECTIVE DEVICE SPACE(S) FOR FUTURE EV CHARGING PURPOSES AS "EV CAPABLE"
 - THE RACEWAY TERMINATION LOCATION IN AS "EV CAPABLE"

Electrical Legend

DUPLEX OUTLET	HIGH EFFICACY RECESSED LIGHTING
EXTERIOR WALL SCONCE LIGHT 6" TAL LED	HIGH EFFICACY LIGHT FIXTURE
220 OUTLET	FAN AND LIGHT COMBINATION (50CFM)
WATER PROOF DUPLEX OUTLET W/ G.F.I.	EXHAUST FAN (50CFM)
DUPLEX OUTLET W/ GROUND FAULT INT.	IBC APPROVED SMOKE DETECTOR, PERMANENTLY WIRED W/ BATTERY BACKUP
EXTERIOR FLOOD LIGHT W/ MOTION SENSORS	CARBON MONOXIDE DETECTOR
ELECTRICAL SUB-PANEL	FLUORESCENT LIGHTING W/ FLUORESCENT BULB
FLOOR OR CEILING DUPLEX OUTLET	FORCED GAS OUTLET
SWITCH	HOSE BIB
THREE WAY SWITCH	FLUORESCENT TUBE LIGHT
DIMMER SWITCH	
AIR SWITCH	
GARBAGE DISPOSAL	

Sheet Title: Electrical Plan


Plan Prepared by: Fernando Martinez

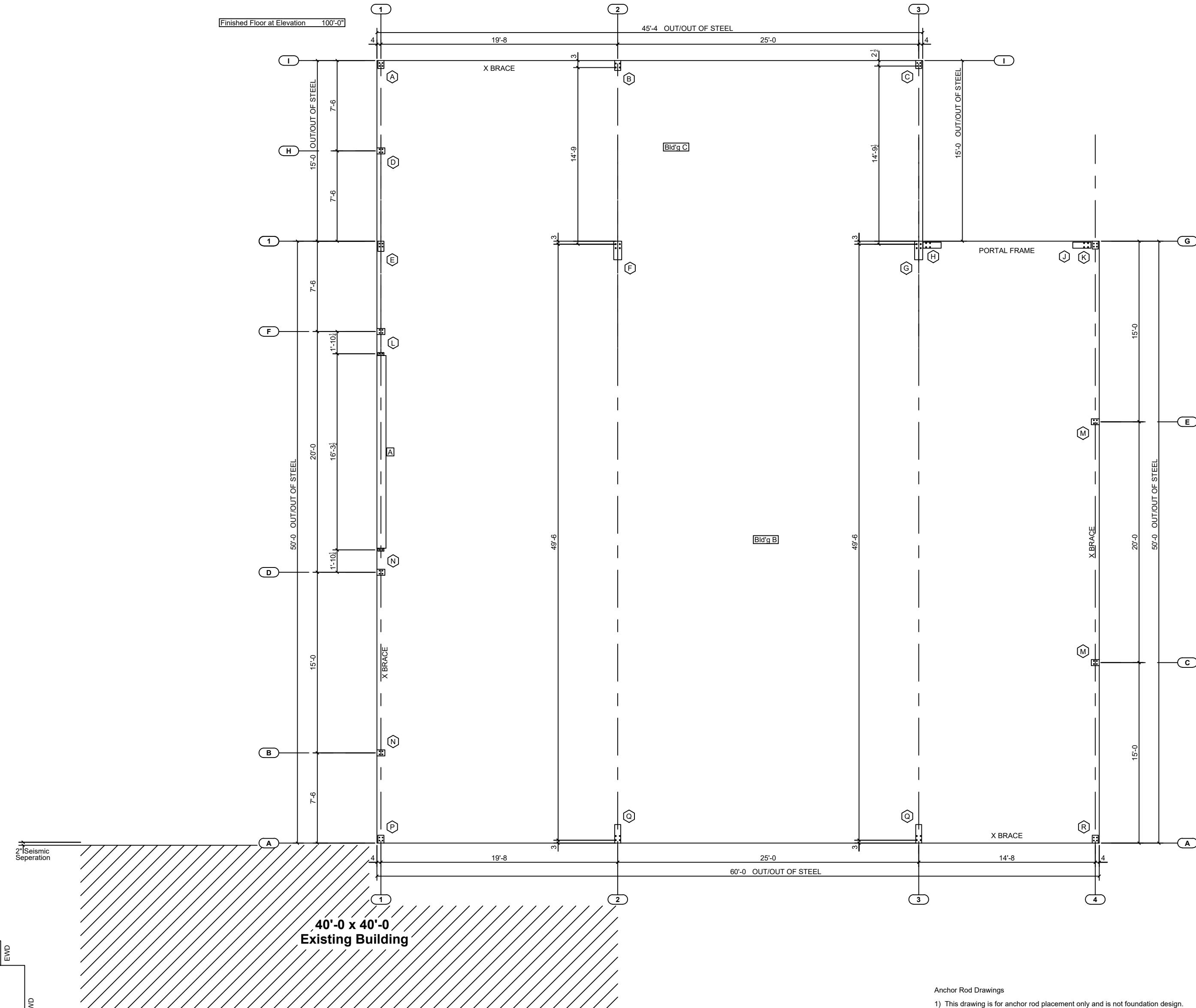
Date: 12/15/25

Scale: 3/16" = 1'-0"

Sheet:

E-1





Anchor Rod Setting Plan

Anchor Rod Drawings

- This drawing is for anchor rod placement only and is not foundation design.
- Foundation must be square and level with all anchor rods true in size, location, and projection.
- Projection shown must be held to keep threads clear of finished concrete.
- This structural design data includes magnitude and location of design loads and support conditions, material properties, and type and size of major structural members. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be superseded and voided by any future mailing.
- Anchor rod size as noted on the drawings has been determined by shear and tension analysis. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer.
- Anchor rods are not provided by the manufacturer. DRAWINGS ARE NOT BE USED FOR CONSTRUCTION.
- 3000 psi concrete compressive strength and marked as ISSUED FOR CONSTRUCTION.

Oct 09, 2025

This item has been electronically signed and sealed by Anan Almughrabi, P.E. on the date and/or time stamp shown using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified by a 3rd Party Certificate Authority on any electronic copy.

EMPIRE STEEL BUILDINGS		Revision	Date	Description	By	Cr'd
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Customer: EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD SITE 300 SAN DIEGO, CA 92121-1761 US		Project Name & Location:				
Drawing Status: <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit		Issued For Approval: <input type="checkbox"/> (Not For Construction) <input checked="" type="checkbox"/> Issued For Construction				
Scale: NOT TO SCALE		Drawn by: TJT	10/2/25	Checked by: CNG	10/3/25	Project Engineer:
Job Number: 20-B-65935		Sheet Number: F1 of 4				
The engineer whose seal appears hereon is employed by or is associated with or provides engineering services for the manufacturer, Cornerstone Building Brands or one of its affiliates, for the products described herein. Said seal or certification is limited to the products described and manufactured by manufacturer only. The undersigned engineer is the overall engineer of record for this project.		Customer:				
ANAN ALMUGHRABI, P.E. CALIFORNIA P.E. C053882		Project Name & Location:				
Drawing Status: <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit		Issued For Approval: <input type="checkbox"/> (Not For Construction) <input checked="" type="checkbox"/> Issued For Construction				



Revision	Date	Description	By	Cr'd

EMPIRE STEEL BUILDINGS
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121
(800) 905-3443

Project Name & Location:

KEENE BREWER
5230 CARROLL CANYON RD SITE 300
FALLBROOK, CA 92028-9587 US

Issued For Construction

Issued For Approval

Issued For Permit

Customer:
EMPIRE STEEL BUILDINGS
5230 CARROLL CANYON RD SITE 300
SAN DIEGO, CA 92121-1761 US

Scale: NOT TO SCALE

Drawn by: TJT 10/2/25

Checked by: CNG 10/3/25

Project Engineer:

Job Number: 20-B-65935

Sheet Number: F2 of 4

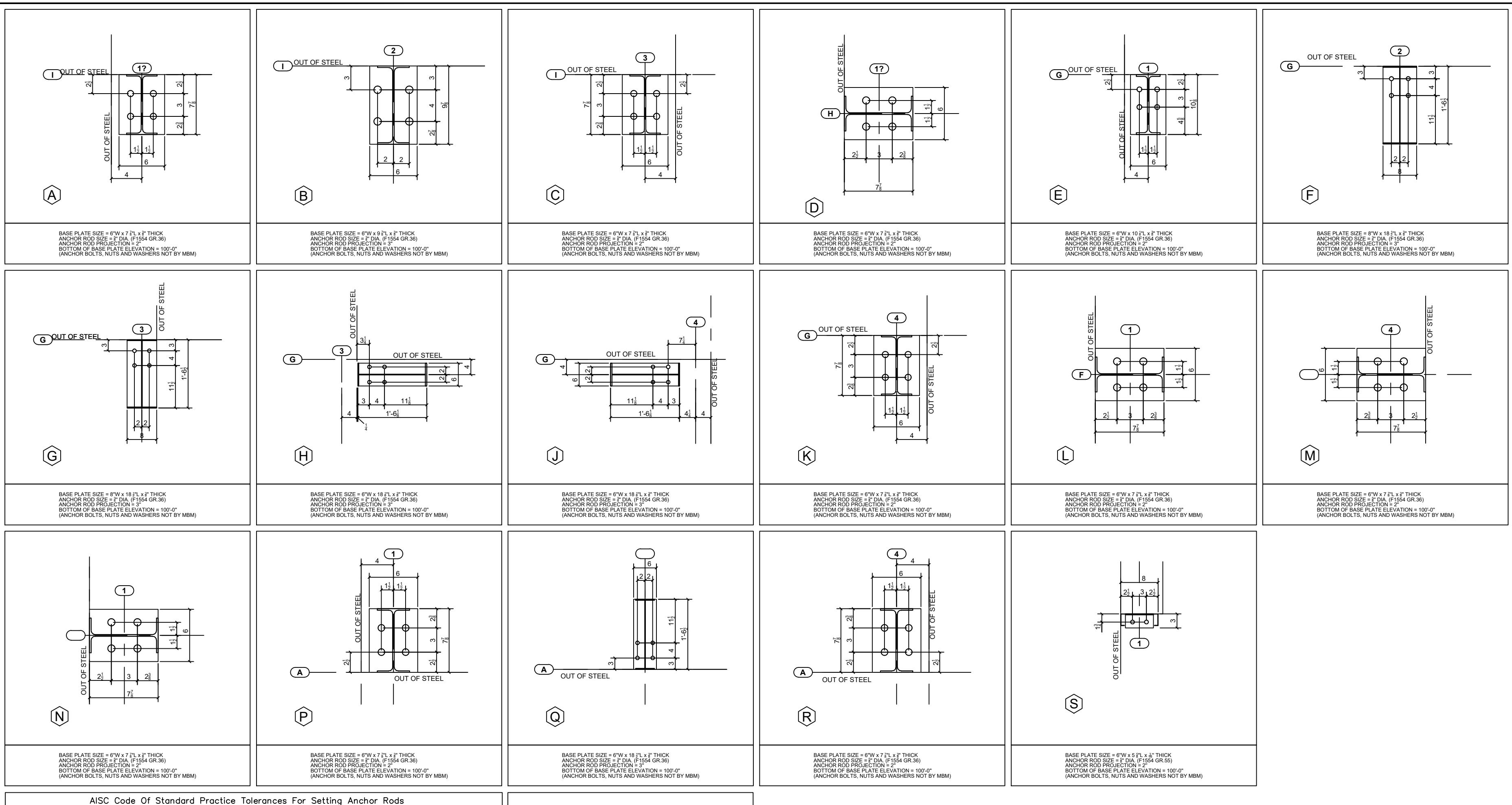
The engineer whose seal appears hereon is employed by or is associated with a provider of engineering services for the manufacturer, Cornerstone Building Brands or one of its affiliates, for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

ANAN ALMUGHABI, P.E.
CALIFORNIA P.E. C053882

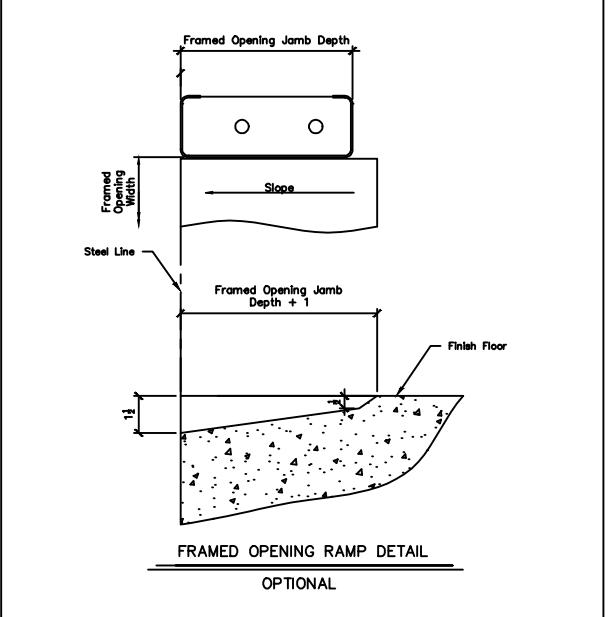
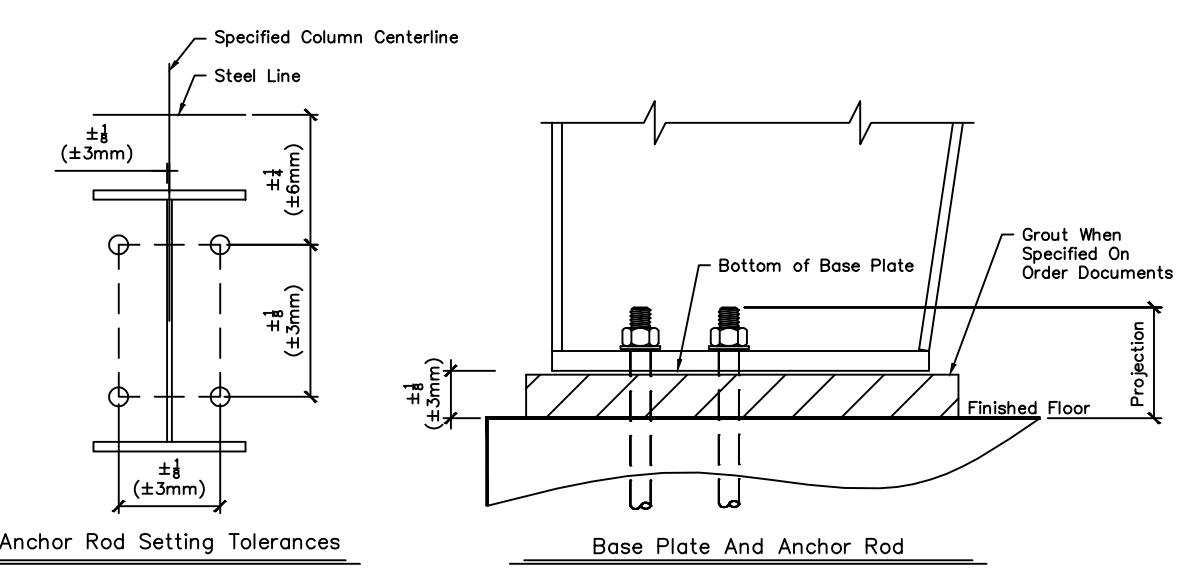


Oct 09, 2025

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AISC Code of Standard Practice Tolerances For Setting Anchor Rods



THESE DRAWINGS SHALL NOT BE USED FOR
CONSTRUCTION PURPOSES UNLESS SEALED
AND MARKED AS "ISSUED FOR CONSTRUCTION"

<div style="border: 1px solid black; padding: 5px;"> <p>FRAME DESCRIPTION: Endwall EW9 USER NAME:Rafael.a JOB NAME:65935B DATE:09/25/25 PAGE:EW-1</p> <p>PATH: R:\jobs\Active\Eng\20-B-65935\ver01-rafael.acerabandan\BLDG-B\run01</p> <p>SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.</p> <p>REACTION NOTATIONS</p> <p>LOAD GROUP REACTION TABLE</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>COLUMN</th> <th>1-G</th> <th>1-F</th> <th>1-D</th> <th>1-B</th> <th>1-A</th> </tr> </thead> <tbody> <tr> <td>LOAD GROUP</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> </tr> <tr> <td>D</td> <td>0.0</td> <td>0.4</td> <td>0.</td> <td>0.8</td> <td>0.</td> <td>0.</td> <td>0.8</td> <td>0.</td> <td>0.</td> <td>0.5</td> <td>0.</td> <td>0.0</td> <td>0.2</td> <td>0.</td> </tr> <tr> <td>C</td> <td>0.0</td> <td>0.2</td> <td>0.</td> <td>0.1</td> <td>0.</td> <td>0.</td> <td>1.2</td> <td>0.</td> <td>0.</td> <td>0.7</td> <td>0.</td> <td>0.0</td> <td>0.2</td> <td>0.</td> </tr> <tr> <td>L</td> <td>0.0</td> <td>0.6</td> <td>0.</td> <td>0.</td> <td>3.5</td> <td>0.0</td> <td>0.</td> <td>3.8</td> <td>0.0</td> <td>0.</td> <td>2.3</td> <td>0.0</td> <td>0.0</td> <td>0.5</td> <td>0.</td> </tr> <tr> <td>W+</td> <td>0.0</td> <td>1.3</td> <td>0.</td> <td>0.</td> <td>-6.4</td> <td>2.5</td> <td>0.</td> <td>-6.1</td> <td>3.1</td> <td>0.</td> <td>-2.7</td> <td>1.8</td> <td>0.0</td> <td>-1.1</td> <td>0.6</td> </tr> <tr> <td>W-</td> <td>0.0</td> <td>-1.1</td> <td>-1.5</td> <td>0.</td> <td>-6.4</td> <td>-2.7</td> <td>0.</td> <td>-6.1</td> <td>-3.4</td> <td>0.</td> <td>-2.7</td> <td>-2.0</td> <td>0.0</td> <td>-1.1</td> <td>-0.7</td> </tr> <tr> <td>WR</td> <td>0.0</td> <td>1.1</td> <td>0.</td> <td>0.</td> <td>-6.4</td> <td>0.0</td> <td>0.</td> <td>-1.6</td> <td>0.0</td> <td>0.</td> <td>3.6</td> <td>-7.2</td> <td>0.0</td> <td>0.0</td> <td>0.</td> </tr> <tr> <td>WL</td> <td>0.0</td> <td>-1.1</td> <td>0.</td> <td>0.</td> <td>-6.4</td> <td>0.0</td> <td>-3.9</td> <td>-10.3</td> <td>0.0</td> <td>0.</td> <td>1.5</td> <td>0.0</td> <td>0.0</td> <td>-1.1</td> <td>0.</td> </tr> <tr> <td>E+</td> <td>0.</td> <td>0.</td> <td>0.1</td> <td>0.</td> <td>0.</td> <td>0.1</td> <td>0.</td> <td>0.</td> <td>0.2</td> <td>0.</td> <td>0.</td> <td>0.1</td> <td>0.</td> <td>0.</td> <td>0.0</td> </tr> <tr> <td>E-</td> <td>0.</td> <td>0.</td> <td>-0.1</td> <td>0.</td> <td>0.</td> <td>-0.1</td> <td>0.</td> <td>0.</td> <td>-0.2</td> <td>0.</td> <td>0.</td> <td>-0.1</td> <td>0.</td> <td>0.</td> <td>0.0</td> </tr> <tr> <td>ER</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>1.9</td> <td>0.</td> <td>1.5</td> <td>-1.9</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>0.</td> </tr> <tr> <td>EL</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>0.</td> <td>-1.5</td> <td>0.</td> <td>0.</td> <td>1.7</td> <td>0.</td> <td>0.</td> <td>1.7</td> <td>0.</td> <td>0.</td> <td>0.</td> </tr> </tbody> </table> <p>LOAD GROUP DESCRIPTION</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>DL</th> <th>Coll</th> <th>PLL1</th> <th>PLL2</th> <th>LL</th> <th>RBDWEQ</th> <th>EQ</th> <th>RBUPEQ</th> <th>W1</th> <th>WL1</th> <th>WL2</th> <th>WL3</th> <th>WL4</th> <th>LWL1</th> <th>RBUPLY</th> <th>LWL2</th> <th>LWL3</th> <th>LWL4</th> <th>RBDWLW</th> </tr> </thead> <tbody> <tr> <td>Roof Dead Load</td> <td>Roof Collateral Load</td> <td>Pattern Live Load Left Leanto/Canopy [PLL1x]</td> <td>Pattern Live Load Right Leanto/Canopy [PLL2x]</td> <td>Roof Live Load</td> <td>Downward Acting Rod Brace Load from Long. 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USER NAME:Rafael.a JOB NAME:65935B DATE:09/30/25 PAGE:2-3 FILE:REW3BLDG1</p> <p>SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.</p> <p>REACTION NOTATIONS</p> <p>LOAD GROUP REACTION TABLE</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>COLUMN</th> <th>2-G</th> <th>2-F</th> <th>2-D</th> <th>2-B</th> <th>2-A</th> </tr> </thead> <tbody> <tr> <td>LOAD GROUP</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> </tr> <tr> <td>D</td> <td>0.7</td> <td>2.6</td> <td>-0.0</td> <td>-0.7</td> <td>2.0</td> <td>-0.0</td> </tr> <tr> <td>COLL</td> <td>1.3</td> <td>4.5</td> <td>-0.0</td> <td>-1.3</td> <td>3.3</td> <td>-0.0</td> </tr> <tr> <td>PLL1</td> <td>-0.1</td> <td>3.7</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>PLL2</td> <td>2.8</td> <td>9.8</td> <td>-0.0</td> <td>-2.8</td> <td>8.7</td> <td>-0.0</td> </tr> <tr> <td>RBDWEQ</td> <td>0.0</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>EQ</td> <td>-1.8</td> <td>-1.4</td> <td>-0.0</td> <td>-2.3</td> <td>-1.3</td> <td>-0.0</td> </tr> <tr> <td>RBUPEQ</td> <td>0.0</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>W1</td> <td>-4.6</td> <td>-14.3</td> <td>-0.0</td> <td>-0.3</td> <td>-5.9</td> <td>-0.0</td> </tr> <tr> <td>WL2</td> <td>-3.8</td> <td>-8.7</td> <td>-0.0</td> <td>-0.8</td> <td>-2.0</td> <td>-0.0</td> </tr> <tr> <td>WL3</td> <td>-1.4</td> <td>-9.8</td> <td>-0.0</td> <td>6.4</td> <td>-11.8</td> <td>-0.0</td> </tr> <tr> <td>WL4</td> <td>-0.4</td> <td>-5.3</td> <td>-0.0</td> <td>-0.0</td> <td>-0.3</td> <td>-0.0</td> </tr> <tr> <td>LWL1</td> <td>-0.9</td> <td>-11.7</td> <td>-0.0</td> <td>1.7</td> <td>-3.3</td> <td>-0.0</td> </tr> <tr> <td>RBUPLY</td> <td>-0.0</td> <td>-0.0</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>LWL2</td> <td>-2.8</td> <td>-10.3</td> <td>-0.0</td> <td>1.3</td> <td>-10.8</td> <td>-0.0</td> </tr> <tr> <td>LWL3</td> <td>-0.5</td> <td>-1.0</td> <td>-0.0</td> <td>0.0</td> <td>-0.5</td> <td>-0.0</td> </tr> <tr> <td>LWL4</td> <td>-1.7</td> <td>-4.8</td> <td>-0.0</td> <td>0.7</td> <td>-5.9</td> <td>-0.0</td> </tr> <tr> <td>RBDWLW</td> <td>0.0</td> <td>0.0</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> </tbody> </table> </div> <div data-bbox="2640 421 3484 564" data-label="Form"> <p>FRAME ID #22 cs 50-18-219-833 20/110. USER NAME:Rafael.a JOB NAME:65935B DATE:10/01/25 PAGE:2-3 FILE:REW3BLDG1</p> <p>SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.</p> <p>REACTION NOTATIONS</p> <p>LOAD GROUP REACTION TABLE</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>COLUMN</th> <th>3-G</th> <th>3-F</th> <th>3-D</th> <th>3-B</th> <th>3-A</th> </tr> </thead> <tbody> <tr> <td>LOAD GROUP</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> </tr> <tr> <td>DL</td> <td>0.6</td> <td>2.3</td> <td>-0.0</td> <td>-0.6</td> <td>1.8</td> <td>-0.0</td> </tr> <tr> <td>COLL</td> <td>1.1</td> <td>3.5</td> <td>-0.0</td> <td>-1.2</td> <td>3.0</td> <td>-0.0</td> </tr> <tr> <td>PLL1</td> <td>-0.1</td> <td>1.9</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>PLL2</td> <td>2.3</td> <td>7.8</td> <td>-0.0</td> <td>-2.3</td> <td>5.9</td> <td>-0.0</td> </tr> <tr> <td>RBDWEQ</td> <td>-0.1</td> <td>-0.1</td> <td>-0.0</td> <td>0.1</td> <td>4.9</td> <td>-0.0</td> </tr> <tr> <td>EQ</td> <td>-1.4</td> <td>-1.1</td> <td>-0.0</td> <td>-1.8</td> <td>1.1</td> <td>-0.0</td> </tr> <tr> <td>RBUPEQ</td> <td>0.0</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>W1</td> <td>-4.4</td> <td>-12.0</td> <td>-0.0</td> <td>-0.5</td> <td>-5.9</td> <td>-0.0</td> </tr> <tr> <td>WL2</td> <td>-3.4</td> <td>-7.5</td> <td>-0.0</td> <td>-0.7</td> <td>-1.9</td> <td>-0.0</td> </tr> <tr> <td>WL3</td> <td>-1.4</td> <td>-8.2</td> <td>-0.0</td> <td>5.7</td> <td>-10.3</td> <td>-0.0</td> </tr> <tr> <td>WL4</td> <td>-0.4</td> <td>-5.3</td> <td>-0.0</td> <td>-0.0</td> <td>-0.3</td> <td>-0.0</td> </tr> <tr> <td>LWL1</td> <td>-1.2</td> <td>-19.0</td> <td>-0.0</td> <td>-1.4</td> <td>1.0</td> <td>-7.1</td> <td>-0.0</td> </tr> <tr> <td>RBUPLY</td> <td>0.1</td> <td>0.0</td> <td>-0.0</td> <td>-0.1</td> <td>-4.1</td> <td>-3.7</td> <td>-0.0</td> </tr> <tr> <td>LWL2</td> <td>-2.6</td> <td>-8.9</td> <td>-0.0</td> <td>0.7</td> <td>9.3</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>LWL3</td> <td>-0.5</td> <td>-0.5</td> <td>-0.0</td> <td>0.0</td> <td>-0.5</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>LWL4</td> <td>-1.6</td> <td>-4.4</td> <td>-0.0</td> <td>0.5</td> <td>-5.2</td> <td>-0.0</td> <td>-0.0</td> </tr> <tr> <td>RBDWLW</td> <td>0.0</td> <td>0.0</td> <td>-0.0</td> <td>0.0</td> <td>-0.0</td> <td>-0.0</td> </tr> </tbody> </table> </div> <div data-bbox="63 421 1216 525" data-label="Form"> <p>FRAME ID #4 pf 14-66718.5 main building USER NAME:Rafael.a JOB NAME:65935B DATE:10/01/25 PAGE:4-2 FILE:REW4BLDG1</p> <p>SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.</p> <p>REACTION NOTATIONS</p> <p>LOAD GROUP REACTION TABLE</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>COLUMN</th> <th>4-G</th> <th>4-C</th> <th>4-E</th> <th>4-G</th> </tr> </thead> <tbody> <tr> <td>LOAD GROUP</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> <td>H</td> <td>V</td> <td>L</td> </tr> <tr> <td>D</td> <td>0.0</td> <td>0.3</td> <td>0.</td> <td>0.</td> <td>0.7</td> <td>0.</td> <td>0.0</td> <td>0.3</td> <td>0.</td> </tr> <tr> <td>COLL</td> <td>0.0</td> <td>0.3</td> <td>0.</td> <td>0.</td> <td>0.9</td> <td>0.</td> <td>0.0</td> <td>0.3</td> <td>0.</td> </tr> <tr> <td>PLL1</td> <td>0.0</td> <td>0.9</td> <td>0.</td> <td>0.</td> <td>3.0</td> <td>0.0</td> <td>0.0</td> <td>0.9</td> <td>0.</td> </tr> <tr> <td>PLL2</td> <td>-0.0</td> <td>-5.3</td> <td>3.7</td> <td>0.</td> <td>-4.5</td> <td>2.9</td> <td>0.</td> <td>-5.9</td> <td>3.3</td> <td>0.0</td> </tr> <tr> <td>RBDWEQ</td> <td>0.0</td> <td>-2.8</td> td> </tr> </tbody> </table> </div> <div data-bbox="1343 1710 2340 2363" data-label="Form"> <p>FRAME DESCRIPTION: Endwall EW9 USER NAME:Rafael.a JOB NAME:65935B DATE:10/01/25 PAGE:EW-2</p> <p>PATH: R:\jobs\Active\Eng\20-B-65935\ver01-rafael.acerabandan\BLDG-B\run01</p> <p>SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.</p> <p>REACTION NOTATIONS</p> <p>LOAD GROUP REACTION TABLE</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>COLUMN</th> <th>4-A</th> <th>4-C</th> <th>4-E</th> <th>4-G</th> </tr> </thead> <tbody> <tr> <td>LOAD GROUP</td> <td>H</td> <td>V</td></tr></tbody></table></div>	COLUMN	1-G	1-F	1-D	1-B	1-A	LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L	D	0.0	0.4	0.	0.8	0.	0.	0.8	0.	0.	0.5	0.	0.0	0.2	0.	C	0.0	0.2	0.	0.1	0.	0.	1.2	0.	0.	0.7	0.	0.0	0.2	0.	L	0.0	0.6	0.	0.	3.5	0.0	0.	3.8	0.0	0.	2.3	0.0	0.0	0.5	0.	W+	0.0	1.3	0.	0.	-6.4	2.5	0.	-6.1	3.1	0.	-2.7	1.8	0.0	-1.1	0.6	W-	0.0	-1.1	-1.5	0.	-6.4	-2.7	0.	-6.1	-3.4	0.	-2.7	-2.0	0.0	-1.1	-0.7	WR	0.0	1.1	0.	0.	-6.4	0.0	0.	-1.6	0.0	0.	3.6	-7.2	0.0	0.0	0.	WL	0.0	-1.1	0.	0.	-6.4	0.0	-3.9	-10.3	0.0	0.	1.5	0.0	0.0	-1.1	0.	E+	0.	0.	0.1	0.	0.	0.1	0.	0.	0.2	0.	0.	0.1	0.	0.	0.0	E-	0.	0.	-0.1	0.	0.	-0.1	0.	0.	-0.2	0.	0.	-0.1	0.	0.	0.0	ER	0.	0.	0.	0.	0.	0.	1.9	0.	1.5	-1.9	0.	0.	0.	0.	0.	EL	0.	0.	0.	0.	0.	-1.5	0.	0.	1.7	0.	0.	1.7	0.	0.	0.	DL	Coll	PLL1	PLL2	LL	RBDWEQ	EQ	RBUPEQ	W1	WL1	WL2	WL3	WL4	LWL1	RBUPLY	LWL2	LWL3	LWL4	RBDWLW	Roof Dead Load	Roof Collateral Load	Pattern Live Load Left Leanto/Canopy [PLL1x]	Pattern Live Load Right Leanto/Canopy [PLL2x]	Roof Live Load	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Downward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	Collateral load	Pattern Live Load Left Leanto/Canopy	Pattern Live Load Right Leanto/Canopy	Roof Live Load	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	Live load	Pattern Live Load Left Leanto/Canopy	Pattern Live Load Right Leanto/Canopy	Roof Live Load	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	Wind load as an inward acting pressure	Wind load as an outward acting suction	Wind load as an outward acting suction	Wind load as an outward acting suction	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	Wind load from the right	Wind load from the left	Wind load from the left	Wind load from the left	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	Seismic force acting inward	Seismic force acting outward	Seismic force acting outward	Seismic force acting outward	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	Seismic force from right	Seismic force from left	Seismic force from left	Seismic force from left	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	Seismic force from right	Seismic force from left	Seismic force from left	Seismic force from left	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind	COLUMN	2-G	2-F	2-D	2-B	2-A	LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L	D	0.7	2.6	-0.0	-0.7	2.0	-0.0	COLL	1.3	4.5	-0.0	-1.3	3.3	-0.0	PLL1	-0.1	3.7	-0.0	0.0	-0.0	-0.0	PLL2	2.8	9.8	-0.0	-2.8	8.7	-0.0	RBDWEQ	0.0	-0.0	0.0	-0.0	-0.0	-0.0	EQ	-1.8	-1.4	-0.0	-2.3	-1.3	-0.0	RBUPEQ	0.0	-0.0	0.0	-0.0	-0.0	-0.0	W1	-4.6	-14.3	-0.0	-0.3	-5.9	-0.0	WL2	-3.8	-8.7	-0.0	-0.8	-2.0	-0.0	WL3	-1.4	-9.8	-0.0	6.4	-11.8	-0.0	WL4	-0.4	-5.3	-0.0	-0.0	-0.3	-0.0	LWL1	-0.9	-11.7	-0.0	1.7	-3.3	-0.0	RBUPLY	-0.0	-0.0	-0.0	0.0	-0.0	-0.0	LWL2	-2.8	-10.3	-0.0	1.3	-10.8	-0.0	LWL3	-0.5	-1.0	-0.0	0.0	-0.5	-0.0	LWL4	-1.7	-4.8	-0.0	0.7	-5.9	-0.0	RBDWLW	0.0	0.0	-0.0	0.0	-0.0	-0.0	COLUMN	3-G	3-F	3-D	3-B	3-A	LOAD GROUP	H	V	L	H	V	L	H	V	L	DL	0.6	2.3	-0.0	-0.6	1.8	-0.0	COLL	1.1	3.5	-0.0	-1.2	3.0	-0.0	PLL1	-0.1	1.9	-0.0	0.0	-0.0	-0.0	PLL2	2.3	7.8	-0.0	-2.3	5.9	-0.0	RBDWEQ	-0.1	-0.1	-0.0	0.1	4.9	-0.0	EQ	-1.4	-1.1	-0.0	-1.8	1.1	-0.0	RBUPEQ	0.0	-0.0	0.0	-0.0	-0.0	-0.0	W1	-4.4	-12.0	-0.0	-0.5	-5.9	-0.0	WL2	-3.4	-7.5	-0.0	-0.7	-1.9	-0.0	WL3	-1.4	-8.2	-0.0	5.7	-10.3	-0.0	WL4	-0.4	-5.3	-0.0	-0.0	-0.3	-0.0	LWL1	-1.2	-19.0	-0.0	-1.4	1.0	-7.1	-0.0	RBUPLY	0.1	0.0	-0.0	-0.1	-4.1	-3.7	-0.0	LWL2	-2.6	-8.9	-0.0	0.7	9.3	-0.0	-0.0	LWL3	-0.5	-0.5	-0.0	0.0	-0.5	-0.0	-0.0	LWL4	-1.6	-4.4	-0.0	0.5	-5.2	-0.0	-0.0	RBDWLW	0.0	0.0	-0.0	0.0	-0.0	-0.0	COLUMN	4-G	4-C	4-E	4-G	LOAD GROUP	H	V	L	H	V	L	H	V	L	D	0.0	0.3	0.	0.	0.7	0.	0.0	0.3	0.	COLL	0.0	0.3	0.	0.	0.9	0.	0.0	0.3	0.	PLL1	0.0	0.9	0.	0.	3.0	0.0	0.0	0.9	0.	PLL2	-0.0	-5.3	3.7	0.	-4.5	2.9	0.	-5.9	3.3	0.0	RBDWEQ	0.0	-2.8	COLUMN	4-A	4-C	4-E	4-G	LOAD GROUP	H	V
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Roof Dead Load	Roof Collateral Load	Pattern Live Load Left Leanto/Canopy [PLL1x]	Pattern Live Load Right Leanto/Canopy [PLL2x]	Roof Live Load	Downward Acting Rod Brace Load from Long. Seismic	Lateral Seismic Load [parallel to plane of frame]	Upward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with +Gcp1	Downward Acting Rod Brace Load from Long. Seismic	Wind from Left to Right with -Gcp1	Wind from Right to Left with +Gcp1	Wind from Right to Left with -Gcp1	Wind from Left to Right with -Gcp1	Wind from Left to Right with +Gcp1	Upward Acting Rod Brace Load from Long. Wind	Windward Corner Right with +Gcp1	Windward Corner Left with -Gcp1	Windward Corner Right with -Gcp1	Downward Acting Rod Brace Load from Long. Wind																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
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LWL4	-1.7	-4.8	-0.0	0.7	-5.9	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
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COLUMN	3-G	3-F	3-D	3-B	3-A																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	
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DL	0.6	2.3	-0.0	-0.6	1.8	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
COLL	1.1	3.5	-0.0	-1.2	3.0	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
PLL1	-0.1	1.9	-0.0	0.0	-0.0	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
PLL2	2.3	7.8	-0.0	-2.3	5.9	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
RBDWEQ	-0.1	-0.1	-0.0	0.1	4.9	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
EQ	-1.4	-1.1	-0.0	-1.8	1.1	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
RBUPEQ	0.0	-0.0	0.0	-0.0	-0.0	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
W1	-4.4	-12.0	-0.0	-0.5	-5.9	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
WL2	-3.4	-7.5	-0.0	-0.7	-1.9	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
WL3	-1.4	-8.2	-0.0	5.7	-10.3	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
WL4	-0.4	-5.3	-0.0	-0.0	-0.3	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																
LWL1	-1.2	-19.0	-0.0	-1.4	1.0	-7.1	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
RBUPLY	0.1	0.0	-0.0	-0.1	-4.1	-3.7	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
LWL2	-2.6	-8.9	-0.0	0.7	9.3	-0.0	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
LWL3	-0.5	-0.5	-0.0	0.0	-0.5	-0.0	-0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																															
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LOAD GROUP	H	V	L	H	V	L	H	V	L																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
D	0.0	0.3	0.	0.	0.7	0.	0.0	0.3	0.																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
COLL	0.0	0.3	0.	0.	0.9	0.	0.0	0.3	0.																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
PLL1	0.0	0.9	0.	0.	3.0	0.0	0.0	0.9	0.																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
PLL2	-0.0	-5.3	3.7	0.	-4.5	2.9	0.	-5.9	3.3	0.0																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																												
RBDWEQ	0.0	-2.8																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																				
COLUMN	4-A	4-C	4-E	4-G																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
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FRAME DESCRIPTION: Endwall EWB **USER NAME:**Rafael.a
PATH: R:\jobs\Active\Eng\20-B-65935\ver01-rafael.arc\cerabadan\BLDG-B\run01 **JOB NAME:**65935B **DATE:**09/25/25
SUPPORT REACTIONS FOR EACH LOAD GROUP **FILE:**REW3BLDG2 **PAGE:**EW-3
NOTE: All reactions are in kips and kip-ft. **TIME:**16:47:51

FRAME ID #1 R: 15/16,-27.917 20/110/**USER NAME:**Rafael.acerabadi **DATE:**09/30/25 **PAGE:**1-2
JOB NAME:65935B **FILE:**itfral.2.fra

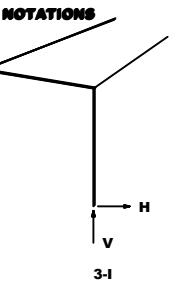
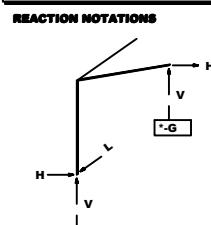
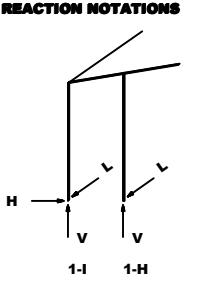
SUPPORT REACTIONS FOR EACH LOAD GROUP

***LOCATION:** 0.000ft; **2**

NOTES:(1) All reactions are in kips and kipft.
(2) The seismic overstrength factor (Omega) is not included in the "RBDWEQ" and "RBUPEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.000
(3) Primary wind load cases are not concurrent.
(4) X-bracing reactions (RBPUWL and RBUPEQ) are combined with LWL and LEQ groups only.

TIME:13:53:24

FRAME DESCRIPTION: Endwall EWD
USER NAME:Rafael.a
DATE:09/25/25
JOB NAME:659358
FILE:REW4BLDG2
PAGE:EW-4
PATH: R:\jobs\Active\Eng\20-B-65935\ver01-rafael.acerabandan\BLDG-B\run01
SUPPORT REACTIONS FOR EACH LOAD GROUP
NOTE: All reactions are in kips and kip-ft.
TIME:16:47:51



LOAD GROUP REACTION TABLE						
COLUMN	1-I			1-H		
LOAD GROUP	H	V	L	H	V	L
D	0.0	0.3	0.	0.	0.5	0.
C	0.0	0.2	0.	0.	0.6	0.
L	0.0	0.6	0.	0.	1.8	0.0
W+	0.0	-1.7	1.0	0.	-3.4	1.2
W-	0.0	-0.1	0.	0.	-3.4	-1.4
WR	0.0	-0.9	0.	0.	-3.4	0.0
WL	0.0	-0.9	0.	0.	-3.4	0.0
E+	0.	-0.8	1.0	0.	0.	0.1
E-	0.	0.8	0.	0.	0.	-0.1

LOAD GROUP REACTION TABLE GRIDLINES * = 2						
COLUMN	*-1			SUPPORT(*-6)		
LOAD GROUP	H	V	L	H	V	L
DL	0.0	0.8	-0.0	-0.0	0.5	-0.0
COLL	0.0	1.3	-0.0	-0.0	1.2	-0.0
PLL1	0.1	3.3	-0.0	-0.1	3.1	-0.0
LL	0.1	3.5	-0.0	-0.1	3.1	-0.0
RBDWEQ	-0.0	0.8	-0.0	0.0	0.0	-0.0
EQ	0.0	0.1	-0.0	-0.7	-0.1	-0.0
RBUPEQ	0.0	-0.8	-1.0	-0.0	-0.0	-0.0
WL1	-1.1	-4.3	-0.0	0.4	-3.9	-0.0
WL2	-2.4	-2.2	-0.0	-1.8	-2.6	-0.0
LWL1	2.3	-4.9	-0.0	4.2	-3.3	-0.0
RBUPLW	0.0	-0.8	-1.0	-0.0	-0.0	-0.0
LWL2	2.3	-3.3	-0.0	3.6	-1.9	-0.0
LWL3	1.0	-2.8	-0.0	2.0	-2.0	-0.0
LWL4	1.0	-1.2	-0.0	1.5	-0.6	-0.0
WL3	1.9	-3.4	-0.0	3.2	-2.1	-0.0
WL4	0.5	-1.2	-0.0	1.0	-0.8	-0.0
RBDWLW	-0.0	0.8	-0.0	0.0	0.0	-0.0

LOAD GROUP REACTION TABLE			
COLUMN	3-I		
LOAD GROUP	H	V	L
D	0.0	0.5	0.
C	0.0	0.6	0.
L	0.0	2.0	0.
W+	0.1	-2.4	0.
W-	0.1	-2.4	0.
WR	0.1	-2.4	0.
WL	0.1	-2.4	0.

LOAD GROUP DESCRIPTION	
D	: Dead load
C	: Collateral load
L	: Live load
W+	: Wind load as an inward acting pressure
W-	: Wind load as an outward acting suction
WR	: Wind force from the right
WL	: Wind force from the left
E+	: Seismic force acting inward
E-	: Seismic force acting outward

LOAD GROUP DESCRIPTION	
DL	: Roof Dead Load
COLL	: Roof Collateral Load
PLL1	: Pattern Live Load Left Leanto/Canopy [PLL1xx]
LL	: Roof Live Load
RBDWEQ	: Downward Acting Rod Brace Load from Long. Seismic
EQ	: Lateral Seismic Load [parallel to plane of frame]
RBUPEQ	: Upward Acting Rod Brace Load from Long. Seismic
WL1	: Wind from Left to Right with +GCpi
WL2	: Wind from Left to Right with -GCpi
LWL1	: Windward Corner Left with +GCpi
RBUPLW	: Upward Acting Rod Brace Load from Long. Wind
LWL2	: Windward Corner Right with +GCpi
LWL3	: Windward Corner Left with -GCpi
LWL4	: Windward Corner Right with -GCpi
WL3	: Wind from Right to Left with +GCpi
WL4	: Wind from Right to Left with -GCpi
RBDWLW	: Downward Acting Rod Brace Load from Long. Wind

LOAD GROUP DESCRIPTION	
D	: Dead load
C	: Collateral load
L	: Live load
W+	: Wind load as an inward acting pressure
W-	: Wind load as an outward acting suction
WR	: Wind force from the right
WL	: Wind force from the left

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.

2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).

- A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
- RIGID FRAMES
 - GABLED BUILDINGS
 - LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 - INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 - SINGLE SLOPE BUILDINGS
 - LEFT COLUMN IS THE LOW SIDE COLUMN.
 - RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 - INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 - ENDWALLS
 - LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 - INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
- ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
- ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
- X-BRACING
 - ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
 - FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, ρ_0 .
 - FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO $1/f_{s0}^2(0.2)$ IS GREATER THAN 0.45.
- REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
- FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
- FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.
- FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE R_dR_o FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

ADDITIONAL NOTES:
(1) Pattern live or snow load cases are not concurrent with any other live or

Customer:		Project Name & Location:		
EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443		KEENE BREWER 2505 WILT RD FALLBROOK, CA 92028-9587 US		
NOT TO SCALE		Drawing Status: <input type="checkbox"/> Issued For Approval (Not For Construction) <input type="checkbox"/> Issued For Construction		
Scale:		<input checked="" type="checkbox"/> Issued For Permit		
By	Ck'd			

Scale: NOT TO SCALE
Drawn by: TJT 10/2/25
Checked by: CNG 10/3/25
Project Engineer:
Job Number: 20-B-65935
Sheet Number: F4 of 4
The engineer whose seal appears hereon is employed by or is contracted to provide engineering services for the manufacturer, Cornerstone Building Brands or one of its affiliates, for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.
ANAN ALMUGHARABI, P.E.
CALIFORNIA P.E. #052892

CALIFORNIA P.E. C055882

Oct 09, 2025

THESE DRAWINGS SHALL NOT BE USED FOR
CONSTRUCTION PURPOSES UNLESS SEALED
AND MARKED AS "ISSUED FOR CONSTRUCTION"

This item has been electronically signed and sealed by
Nhan Almughrabi, P.E. on the date and/or time stamp
shown using a digital signature. Printed copies of this
document are not considered signed and sealed and the
signature must be verified by a 3rd Party Certificate
Authority on any electronic copy.

EMPIRE STEEL BUILDINGS			
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443			
DESIGN CRITERIA			
PROJECT NOTES			
Building Descriptions			
DEFLECTION CRITERIA			
1/2" DIA. A325 BOLT GRIP TABLE			
Drawing Index			
Customer: Project Name & Location: 5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443 Issued For Construction: <input type="checkbox"/> Issued For Approval: <input type="checkbox"/> Issued For Permit: <input type="checkbox"/> Drawing Status: <input checked="" type="checkbox"/> NOT TO SCALE			
NOT TO SCALE			
Drawn by: TJT 10/2/25 Checked by: CNG 10/3/25 Project Engineer: Job Number: 20-B-65935 Sheet Number: E1 of 17			
NOTICE: THESE DRAWINGS SHALL NOT BE USED FOR CONSTRUCTION PURPOSES UNLESS SEALED AND MARKED AS "ISSUED FOR CONSTRUCTION". NOTICE: This item has been electronically signed and sealed by Anan Almughrabi, P.E. on the date and/or time stamp shown using a digital signature. Printed copies of this document are not considered signed and sealed and the signature must be verified by a 3rd Party Certificate Authority on any electronic copy.			

Builder/Contractor Responsibilities

Drawing Validity: These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings: Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC COSP June 2016 Section 4.4.1)

Code Official Approval: It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Building Erection: The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector (AISC COSP June 2016 Section 7.10.3).

Discrepancies: Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC COSP June 2016 Section 3.3)

Materials by Others: All interface and compatibility of any materials not furnished by the manufacturer are the responsibility and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturer's assumptions will govern.

Modification of the Metal Building from Plans: The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design: The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and/or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)

Shimming: In accordance with Section 6.10 of Chapter 4 Common Industry Practices in the Metal Building Systems Manual, shimming is a normal part of erection and is not subject to claim.

DESIGN CRITERIA

Building Code: 2022 CALIFORNIA BUILDING CODE

Building Risk Category: Normal (Risk Category II)

Roof Dead Load:

- Superimposed: 2.27 psf (Bldg B)
- : 2.19 psf (Bldg C)
- Collateral: 6.00 psf
- (6.00 psf Other)

Roof Live Load: 20.00 psf reduction allowed

Wind:

- Ultimate Wind Speed (Vult) : 110 mph
- Nominal Wind Speed (Vasd) : 85 mph (IBC section 1609.3.1)
- Serviceability Wind Speed : 67 mph
- Ground Elevation Factor : 0.97 (854 ft ASL)
- Wind Exposure Category : C
- Exposure Coefficient (MWFRS) : 0.887
- Enclosure Classification : Enclosed Building
- Induced Wind Coefficient (C0) : 1.18/0.18

Unfactored Wind Loads for components not provided by building manufacturer

Zone 5 Areas (within 6.00' of corner)	: 23.72 psf pressure -31.63 psf suction
Zone 4 Areas (away from corners)	: 23.72 psf pressure -25.70 psf suction

These values are the maximum values required based on a 10 sq ft area.

Components with larger areas may have lower wind loads.

Seismic:

- Seismic Importance Factor (Ie): 1.00
- Seismic Design Category : D
- Soil Site Class: D Stiff Soil (Default)
- S1 : 1.077 g Sds : 0.862 g
- S1 : 0.390 g Sd1 : 0.497 g
- Analysis Procedure : Equivalent Lateral Force
- Column Line (Bldg B): 1 & 4 2-3 SWA & SWC
- Basic Force Resisting System: B3 C4 B3 & C4
- Response Modification Coefficient (R): 3.25 3.50 3.25
- Seismic Response Coefficient (Cs): 0.265 0.246 0.265
- Design Base Shear in Kips (V): 2.47 6.13 9.95
- Column Line (Bldg C): SWC
- Basic Force Resisting System: B3
- Response Modification Coefficient (R): 3.25
- Seismic Response Coefficient (Cs): 0.265
- Design Base Shear in Kips (V): 1.01
- Basic Structural System (from ASCE 7-16 Table 12.2-1): B3 - Ordinary Steel Concentrically Braced Frame
- C4 - Ordinary Steel Moment Frame

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A592, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 min. yield. Round and rectangular HSS conform to ASTM A500 Grade B. Cold formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield. For Canada, material properties conform to CAN/CSA G40.20/G40.21 or equivalent.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5, fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-136.

Bolted joints with A325 Type 1 bolts greater than 1/2" diameter are specified as pre-tensioned joints in accordance with the most recent edition of the RCSC Specification for Structural Joints Using ASTM A325 or A490 Bolts. Pre-tensioning can be accomplished by using the turn-of-nut method of tightening, calibrated wrench, turn-off-type tension-control bolts or direct-tension indicator as acceptable to the Inspecting Agency and Building Official. Installation inspection requirements for pre-tensioned joints (Specification for Structural Joints Section 9.2) using turn-of-nut method is suggested. The connections on this project are not slip critical.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

This metal building system is designed as an Enclosed Building or Enclosed Building, Exterior and/or operable components in accordance with the building code, drawings, and applicable "component" must be designed to withstand the required component and cladding wind pressures specified by the building code. In order to maintain the metal building system's Enclosed Building or Enclosed Building condition, all Components shall be closed when wind velocities reach half the designed wind load for the metal building system as shown on the drawings and design criterias documentation. Failure to maintain the metal building system's Enclosed Building or Enclosed Building condition will violate and void all warranties and certifications applicable to the material supplied by the metal building manufacturer.

The materials by the manufacturer will be fabricated in a facility that has received Certification of Accreditation for the Manufacture of Metal Building Systems (AC472) from International Accreditation Service (IAS). This certification is recognized under Section 1704 of the IBC for approved fabricator.

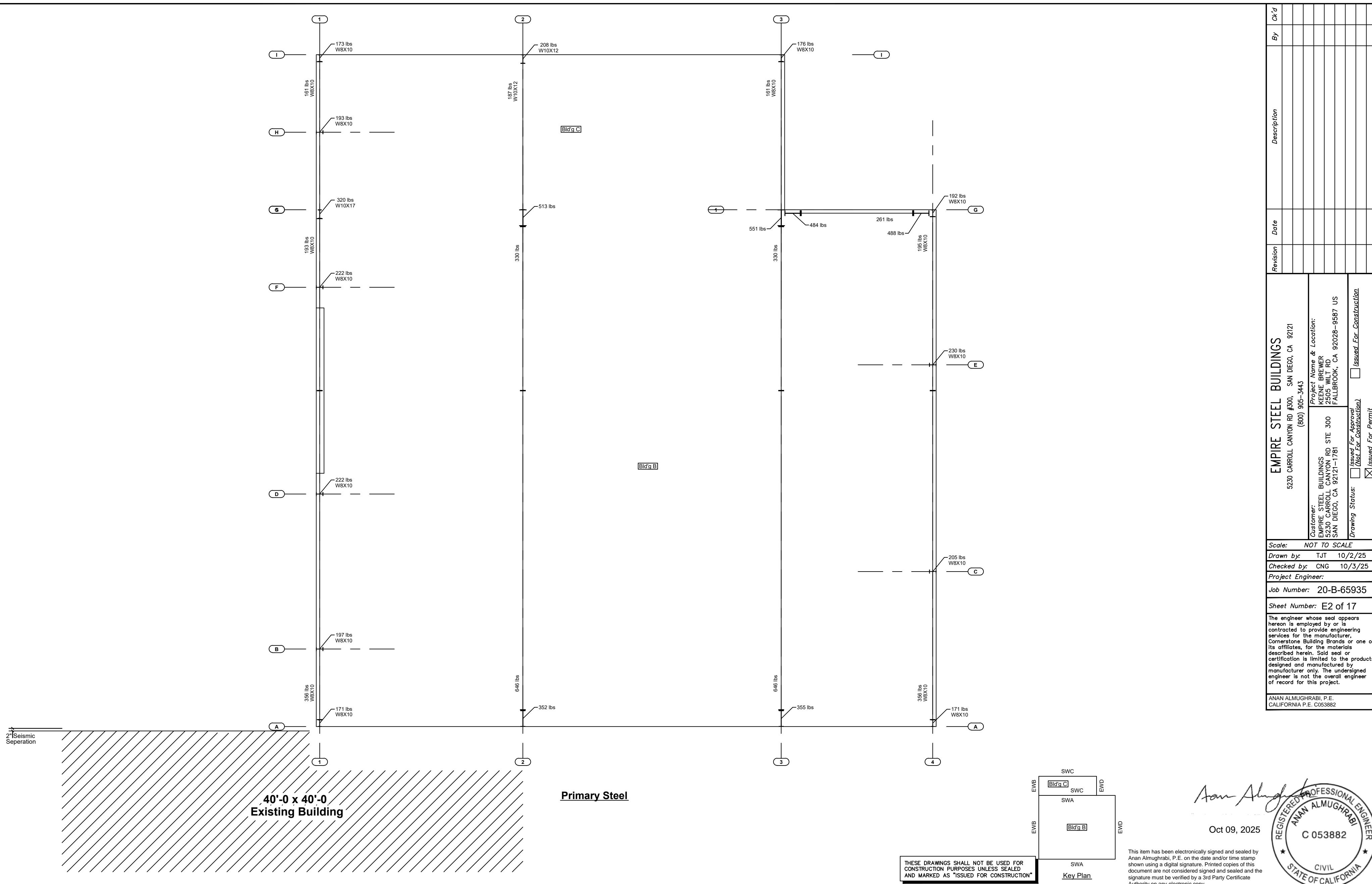
Framed openings, walk doors, and open areas shall be located in the bay and elevations as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

The common wall at the existing building is to remain sheeted.

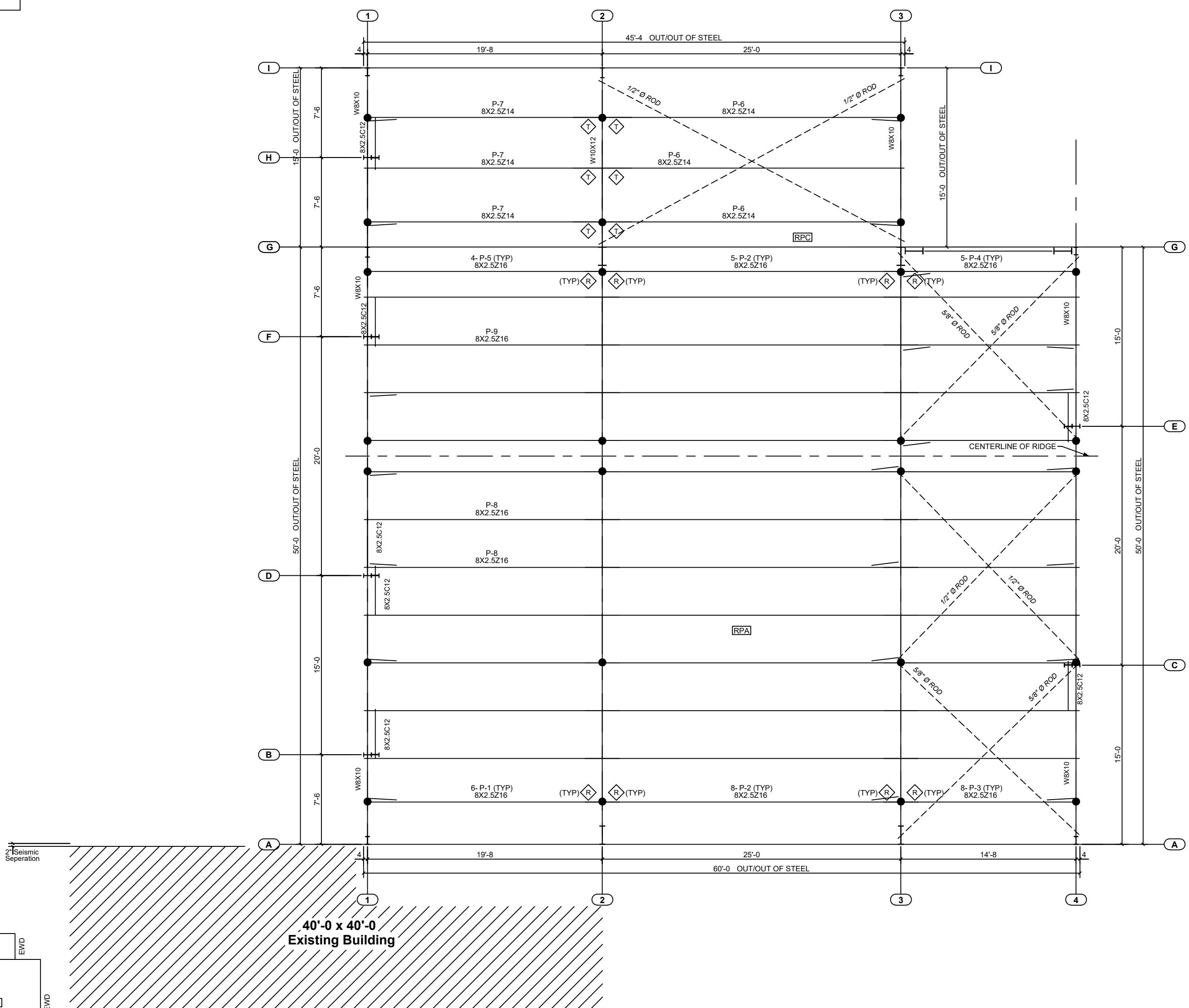
Using 6.9 in X 6.375 in eave gutter with 4 x 5 downspouts, the roof drainage system has been designed using the method outlined in the MBMA Metal Building Systems Manual. Downspout locations have not been located on these drawings. The downspouts are to be placed on the building sidewalls at a spacing not to exceed 60 feet with the first downspout from both ends of the gutter run within 30 feet of the end. Downspout spacing that does not exceed the maximum spacing will be in compliance with the building code. The gutter and downspout system as provided by the manufacturer is designed to accommodate 2.98 in/hr rainfall intensity.

Investigation of the existing structure for possible detrimental effects due to the metal building addition is not within the metal building manufacturer's scope of work. It is strongly recommended that the original designer or other responsible professional be retained to analyze the existing structure, recommending any reinforcement that may be needed. The metal building manufacturer and its certifying engineer expressly exclude the existing structure for any warranty or certification whether written, verbal or implied.

The framing at Bldg B, EWB and EWD is NOT designed to receive a future bay addition. Corresponding frame reactions are calculated based upon actual tributary area. This frame is designed to span clear between exterior columns, as such, the rafter is expected to deflect downward and upward due to vertical loads (i.e. gravity, wind, etc.). When an endwall column is present under the non-expandable frame, the standard top-of-column connection to the rafter will not allow for vertical movement. Therefore, the endwall column is adequately designed as a load bearing. Reactions for the endwall column will reflect vertical loads. Removal of the endwall column under the non-expandable frame is allowed as the frame is adequately designed to span clear between exterior columns under specified vertical deflection limits.



- - DENOTES: CLIP LOCATION
SC90 AT 8" PURLINS
SC92 AT 10" PURLINS
SC94 AT 12" PURLINS



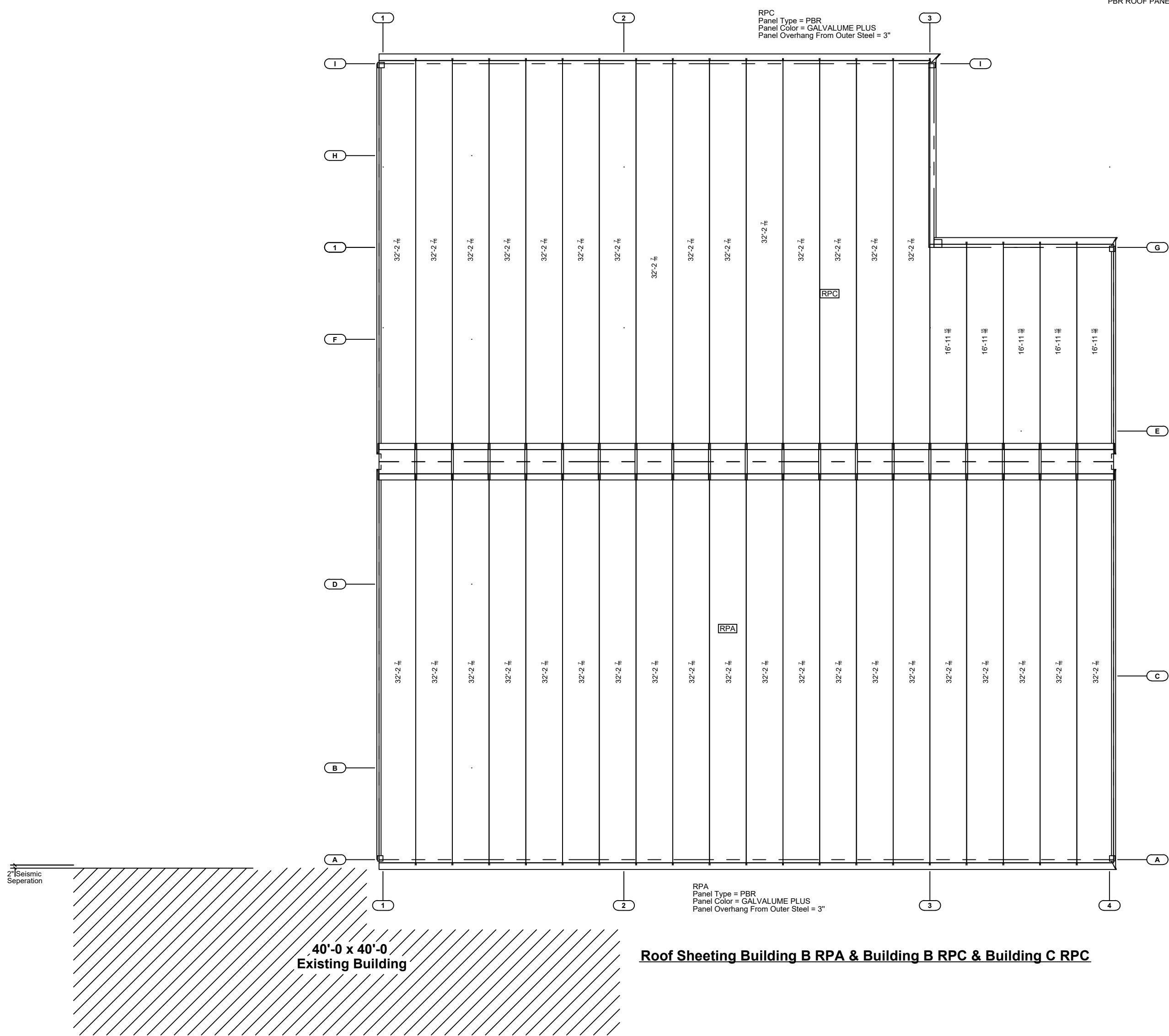
Roof Framing

ZEE SECTION LAP TABLE			
SYMBOL	LAP LENGTH	SYMBOL	LAP LENGTH
	-0'-0 1/4"		2'-5 3/4"
	0'-3 3/4"		3-1 3/4"
	1'-5 3/4"	REFER TO CF01122	

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Oct 09, 2025

A circular registration stamp. The outer ring contains the text "REGISTERED PROFESSIONAL ENGINEER" at the top and "STATE OF CALIFORNIA" at the bottom. The inner circle contains "ANAN ALMUGHARABI" at the top and "C 053882" in the center. Below the center is the word "CIVIL". There are two small stars, one on each side of the center text.



Roof Sheeting Building B RPA & Building B RPC & Building C RPC

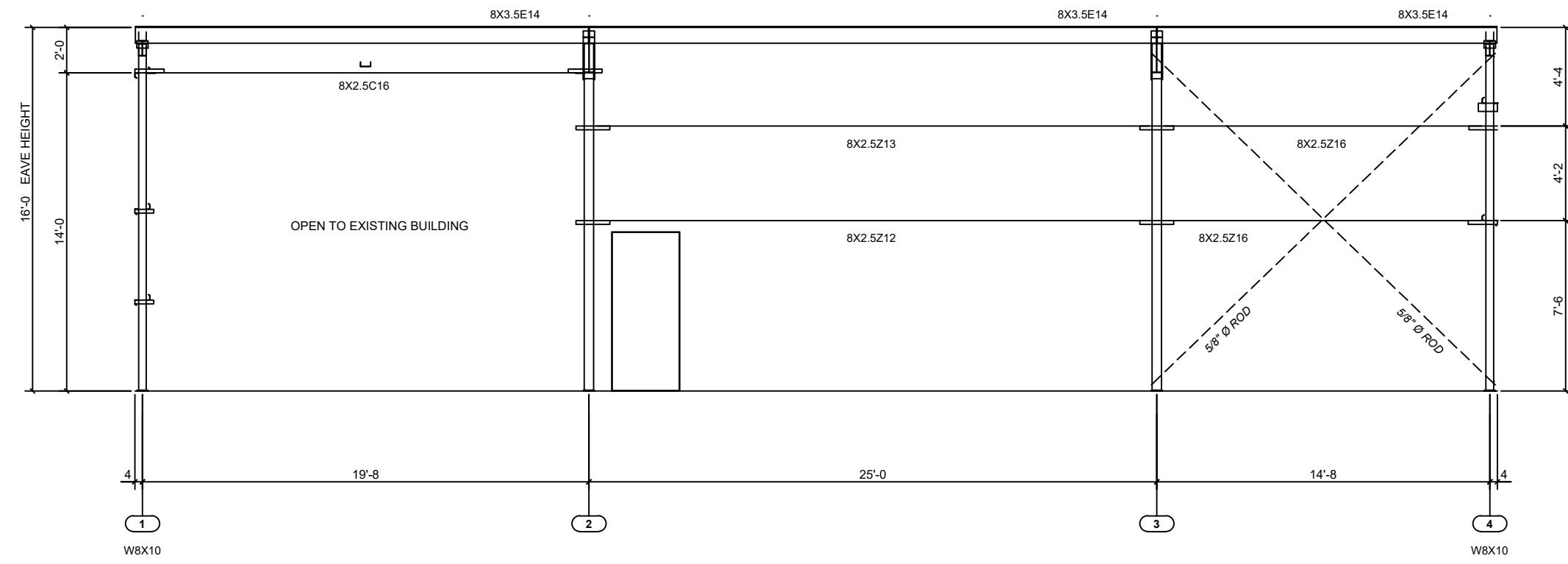
The Key Plan diagram illustrates the layout of buildings A and B relative to SWC and SWA. The diagram consists of a large rectangular area divided into four quadrants by two intersecting lines. The vertical line on the left is labeled 'EWB' (East-West Boundary) and the horizontal line at the top is labeled 'SWC' (South-West Boundary). The top-right quadrant is labeled 'Bldg C' with 'SWC' below it. The bottom-right quadrant is labeled 'Bldg B' with 'SWA' below it. The bottom-left quadrant is labeled 'SWA' at the bottom center. The left boundary is labeled 'EWB' and the top boundary is labeled 'SWC'.

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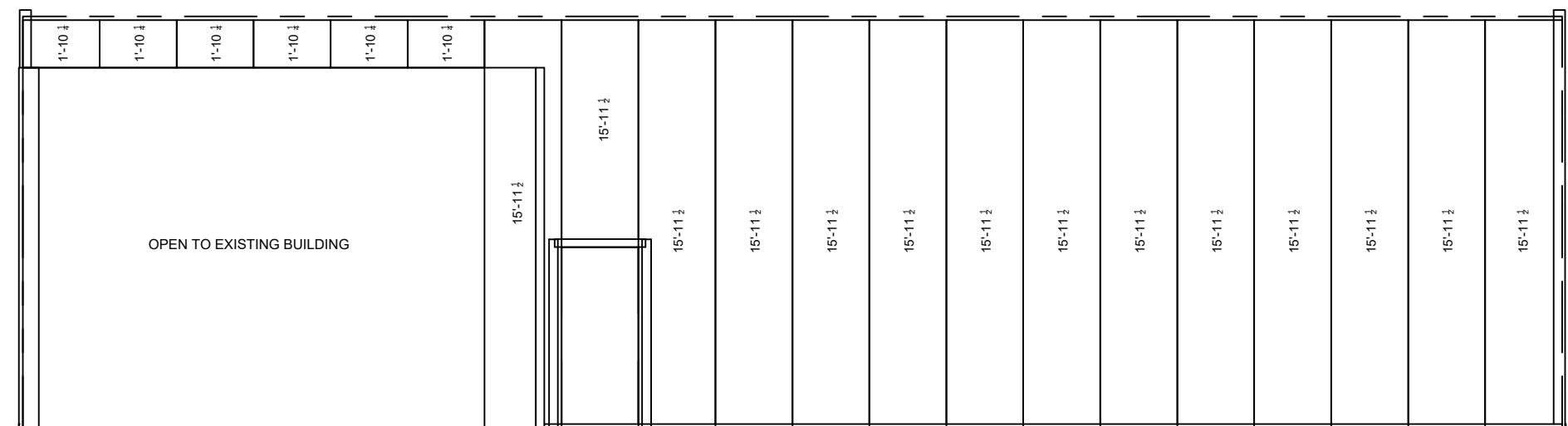
Oct 09 2025

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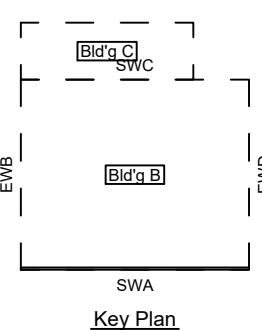


Sidewall Framing SWA at Grid Line A
Building B



Sidewall Sheeting Building B SWA

PBR Wall Panels
Panel Coverage = 3'-0"
Panel Color = S200 POLAR WHITE
Panel Pkg. Req'd. = PBS-4
Field Cut Panel and Trim as
required per Construction Details



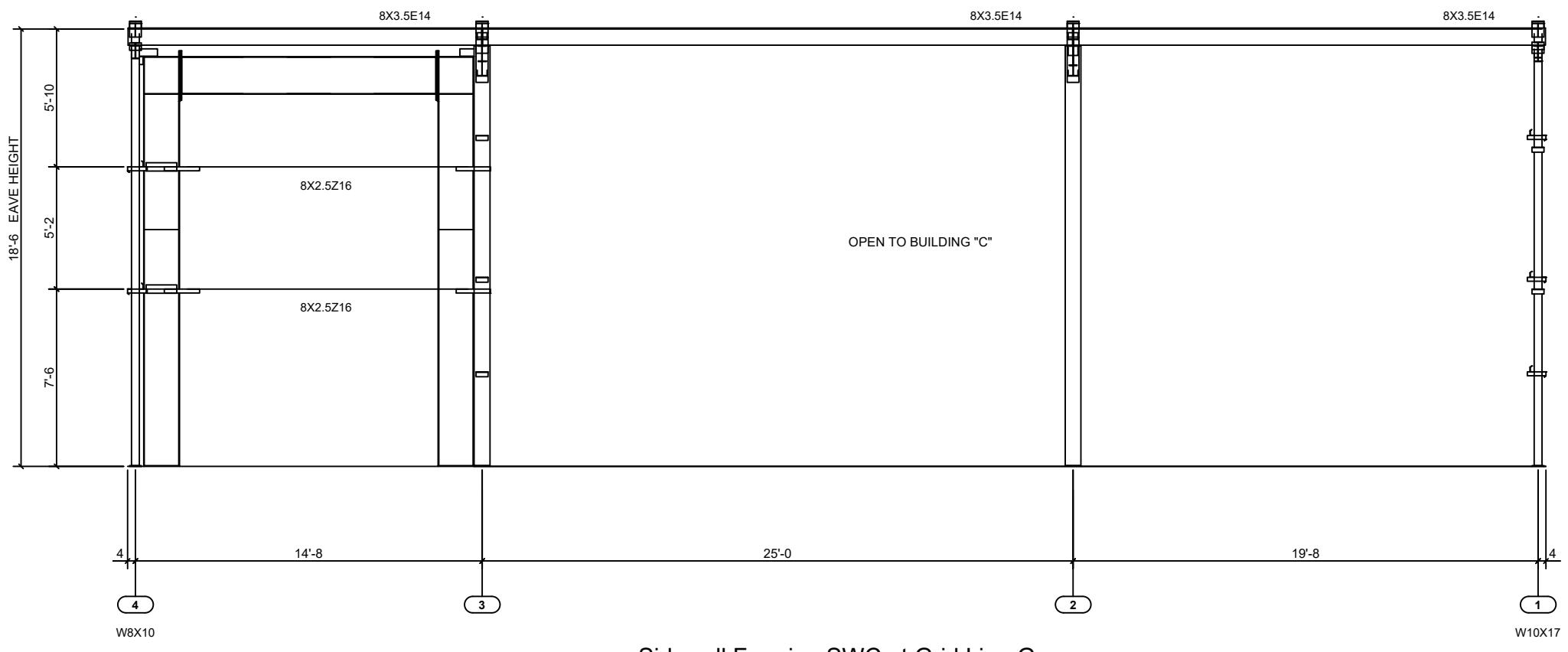
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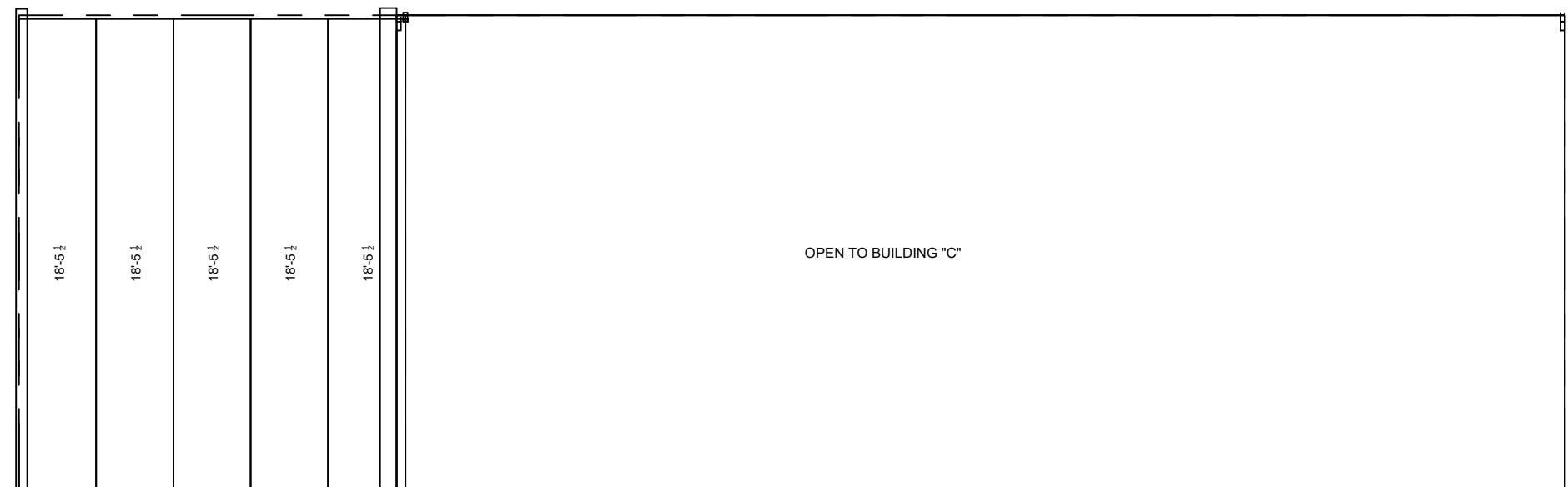
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EMPIRE STEEL BUILDINGS		Revision	Date	Description	By	Cr'd
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Project Name & Location: KEENE BREWER 5230 CARROLL CANYON RD SITE 300 2600 MILIT RD FALLBROOK, CA 92028-9587 US						
Customer: EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD SITE 300 SAN DIEGO, CA 92121-1781						
Drawing Status: <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit						

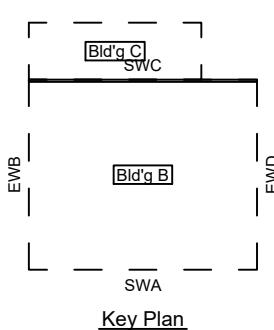




Sidewall Framing SWC at Grid Line G
Building B

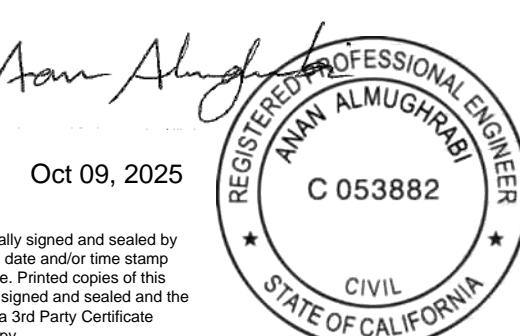


Sidewall Sheeting Building B SWC



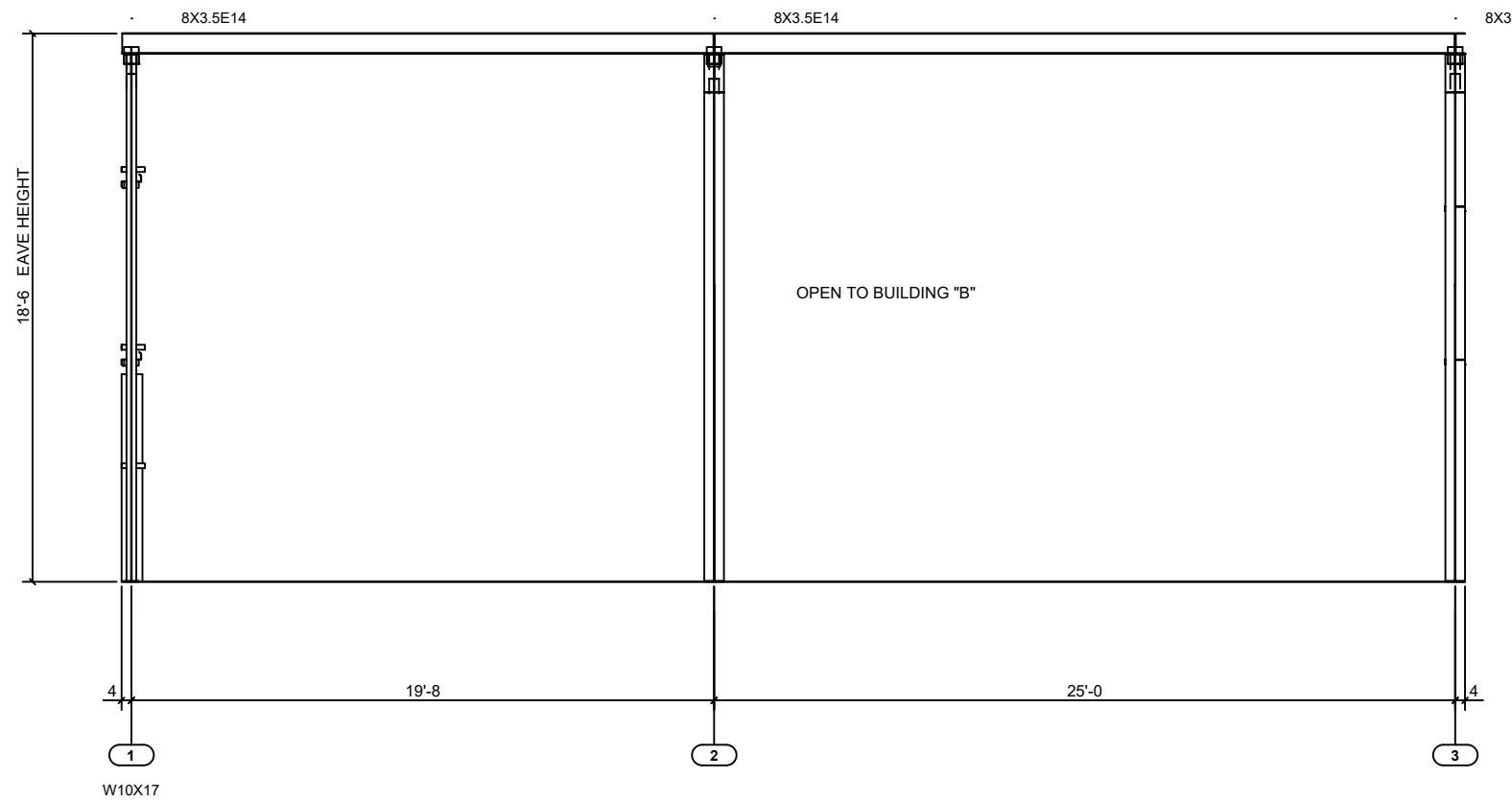
EMPIRE STEEL BUILDINGS		Revision	Date	Description	By	Cr'd
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Project Name & Location: KEENE BREWER 5230 CARROLL CANYON RD SITE 300 2600 MILIT RD FALLBROOK, CA 92028-9587 US						
Customer: EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD SITE 300 2600 MILIT RD SAN DIEGO, CA 92121-1781						
Drawing Status: <input type="checkbox"/> Issued For Approval <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit						
Scale: NOT TO SCALE						
Drawn by: TJT 10/2/25						
Checked by: CNG 10/3/25						
Project Engineer:						
Job Number: 20-B-65935						
Sheet Number: E6 of 17						
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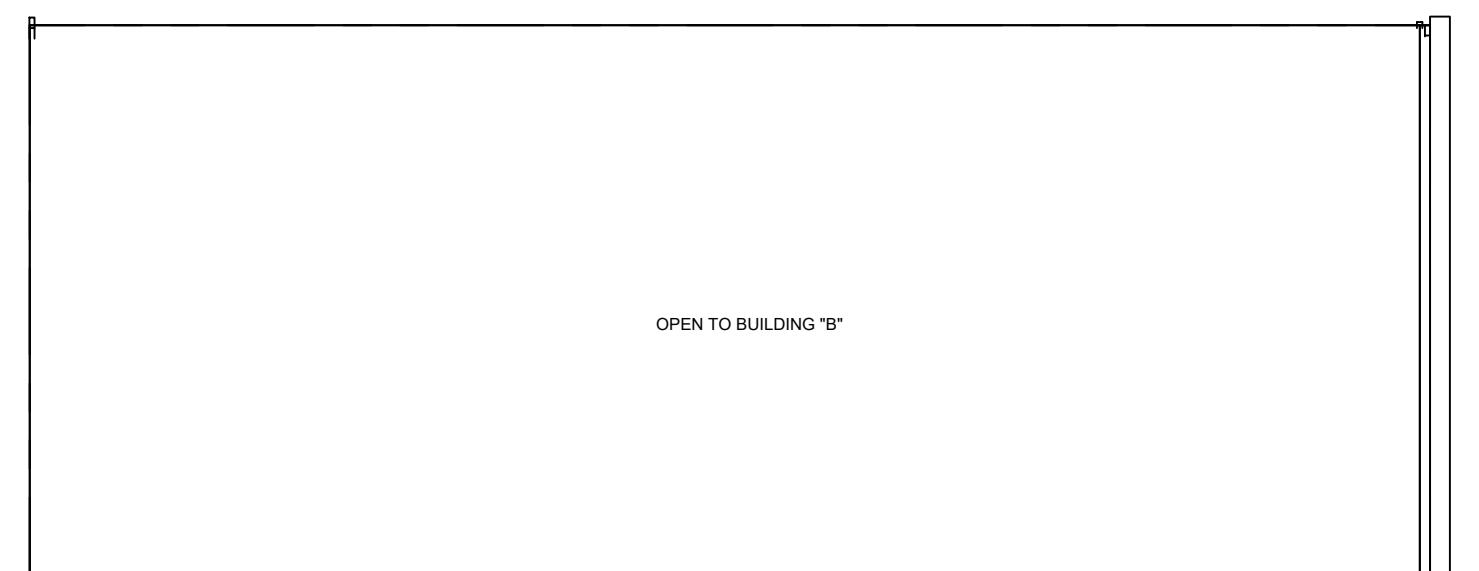


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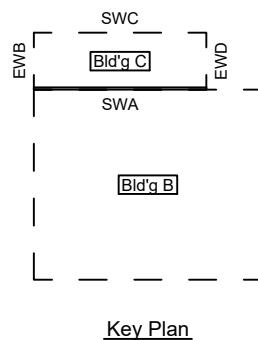
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Sidewall Framing SWA at Grid Line 1
Building C



Sidewall Sheeting Building C SWA



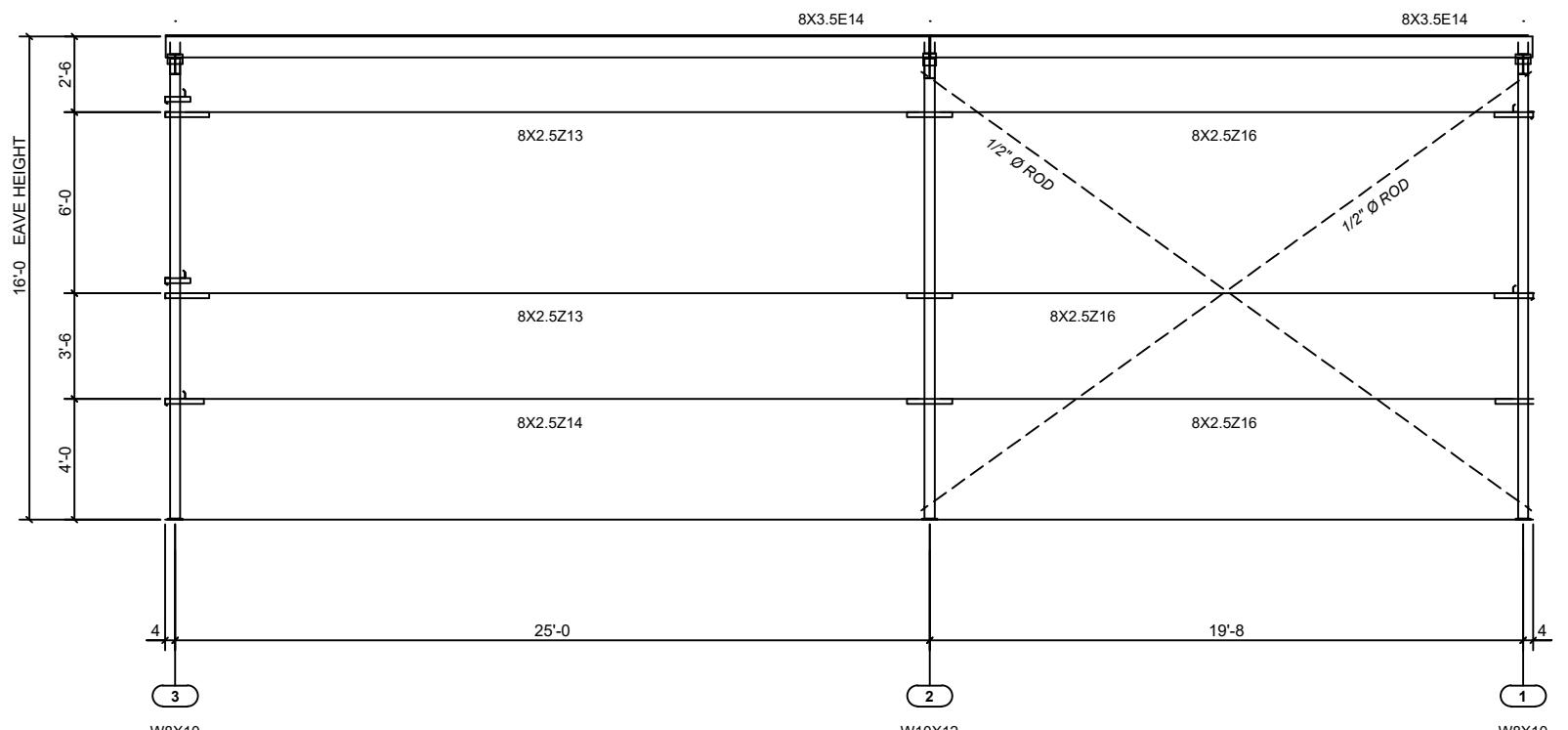
EMPIRE STEEL BUILDINGS		Revision	Date	Description	By	Cr'd
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Customer: EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD SITE 300 SAN DIEGO, CA 92121-1781		Project Name & Location: IEENE BREWER 2600 MILIT RD FALLBROOK, CA 92028-9587 US				
Drawing Status: <input type="checkbox"/> Issued For Approval <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit						
Scale: NOT TO SCALE Drawn by: TJT 10/2/25 Checked by: CNG 10/3/25 Project Engineer: Job Number: 20-B-65935 Sheet Number: E7 of 17 The engineer whose seal appears hereon is employed by or is associated with or provides engineering services for the manufacturer, Cornerstone Building Brands or one of its affiliates, for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.						
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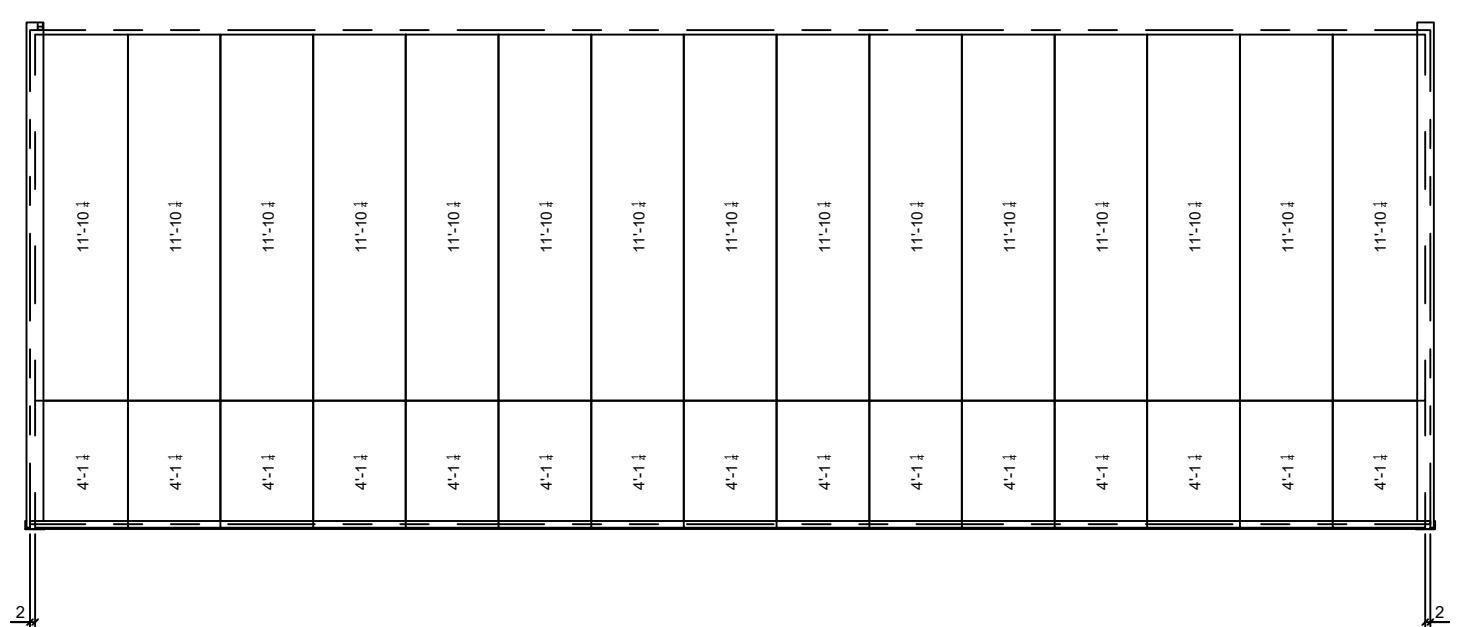
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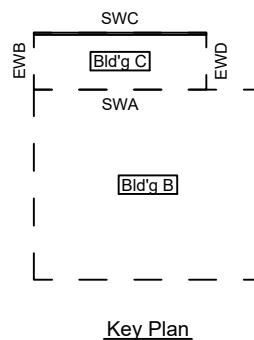
W10X12

Sidewall Framing SWC at Grid Line I

Building C



Sidewall Sheeting Building C SWC

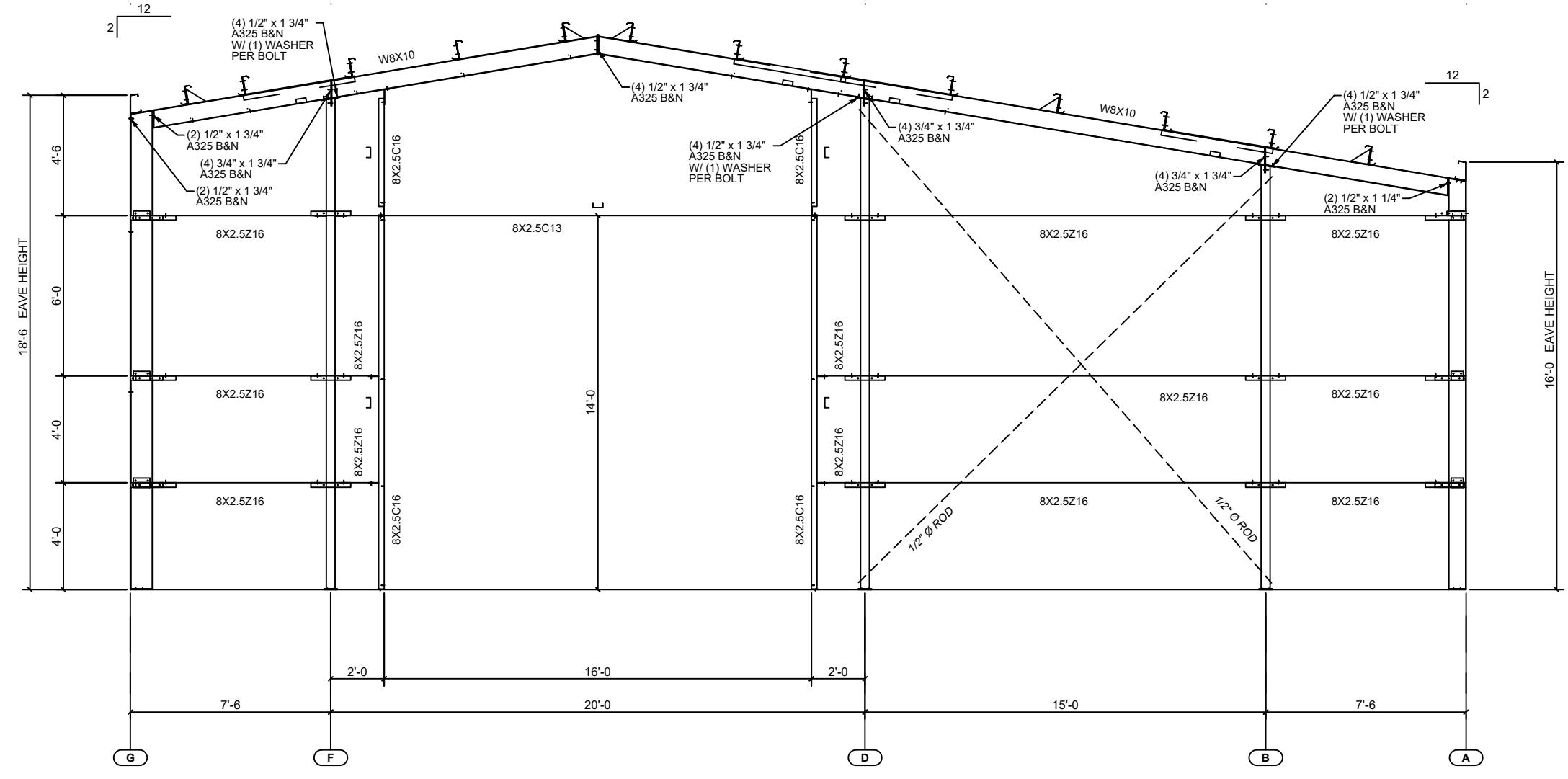


Project Name & Location:		Revision	Date	Description	By	Ck'd
EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Customer:	KEENE BREWER 2505 WILT RD FALLBROOK, CA 92028-9587 US					
Drawing Status:	<input type="checkbox"/> Issued For Approval <input checked="" type="checkbox"/> Not For Construction				<input type="checkbox"/> Issued For Construction	
Scale:	NOT TO SCALE					
Drawn by:	TJT 10/2/25					
Checked by:	CNG 10/3/25					
Project Engineer:						
Job Number:	20-B-65935					
Sheet Number:	E8 of 17					
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Oct 09 2025

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Endwall Framing EWB at Grid Line SL

PBR Wall Panels
Panel Coverage = 3'-0"
Panel Color = S200 POLAR WHITE
Panel Pkg. Req'd. = PBS-1
Field Cut Panel and Trim as
required per Construction Details



required per Construction Details

PBR Wainscot Wall Panels
Panel Coverage = 3'-0"
Panel Color = S200 CHARCOAL GRAY
Panel Pkg. Req'd. = PBS-1
Field Cut Panel and Trim as
required per Construction Details

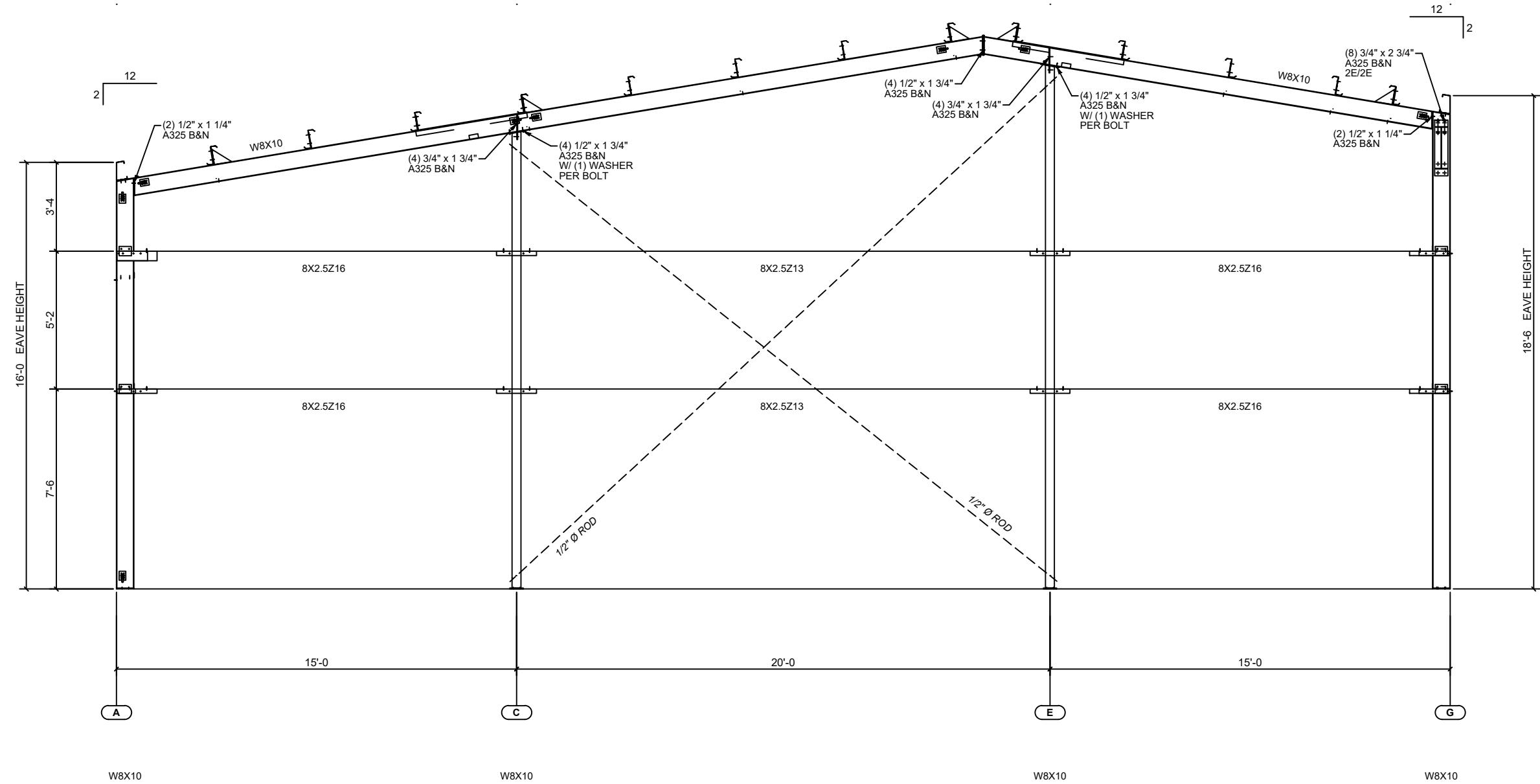
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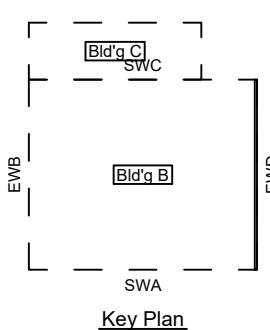
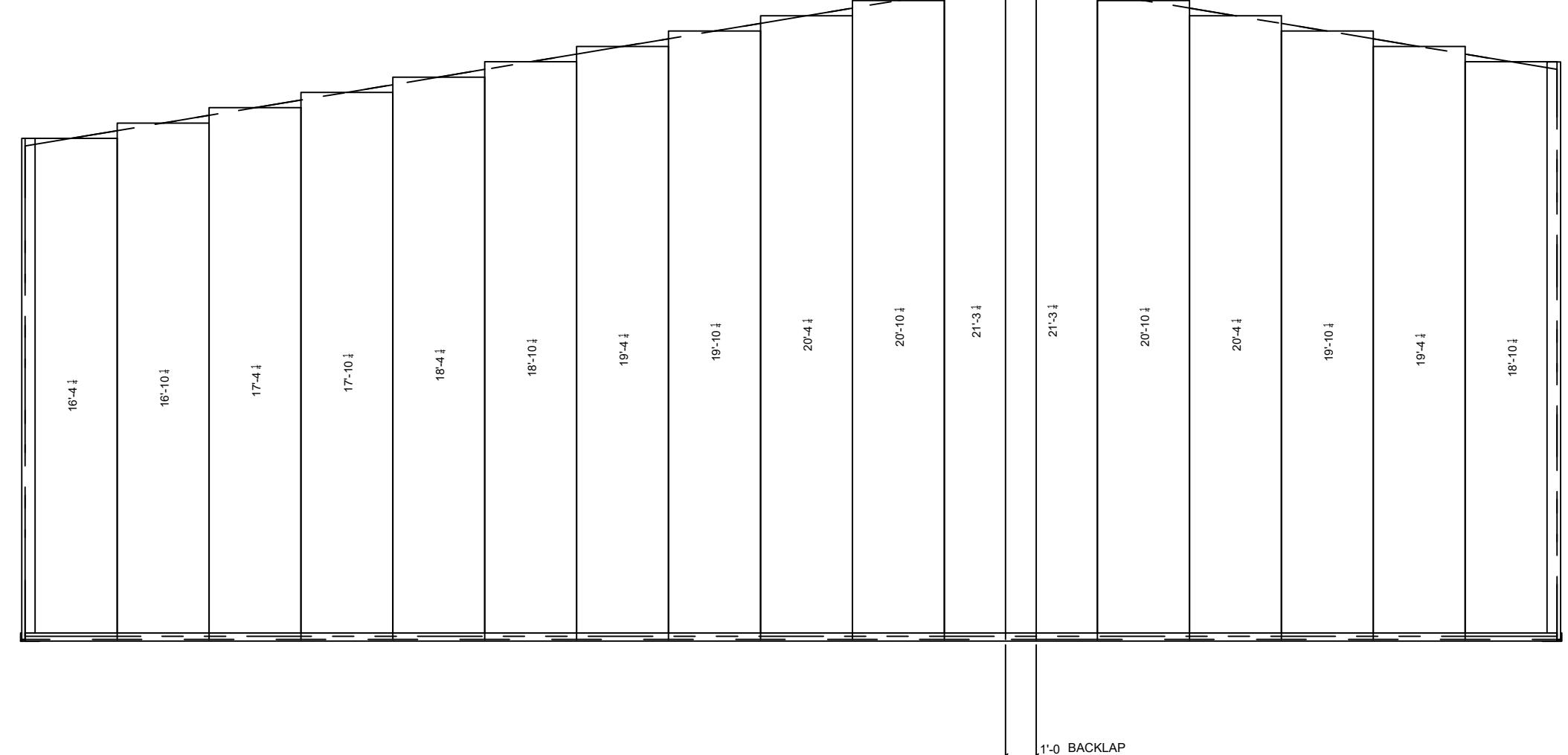
Oct 09, 2025



EMPIRE STEEL BUILDINGS		Revision	Date	Description	By	Ck'd
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Customer: EMPIRE STEEL BUILDINGS 5230 CARROL CANYON RD STE 300 SAN DIEGO, CA 92121-1781		Project Name & Location: KEENE BREWER 2505 WILT RD FALLBROOK, CA 92028-9587 US				
Drawing Status:		<input type="checkbox"/> Issued For Approval <input type="checkbox"/> (Not For Construction)		<input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit		
Scale: NOT TO SCALE						
Drawn by: TJT 10/2/25						
Checked by: CNG 10/3/25						
Project Engineer:						
Job Number: 20-B-65935						
Sheet Number: E9 of 17						
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ANAN ALMUGHRABI, P.E. CALIFORNIA P.E. C053882						



PBR Wall Panels
 Panel Coverage = 3'-0"
 Panel Color = S200 POLAR WHITE
 Panel Redg. = TRS-3
 Field Cut Panels and Trim as
 required per Construction Details



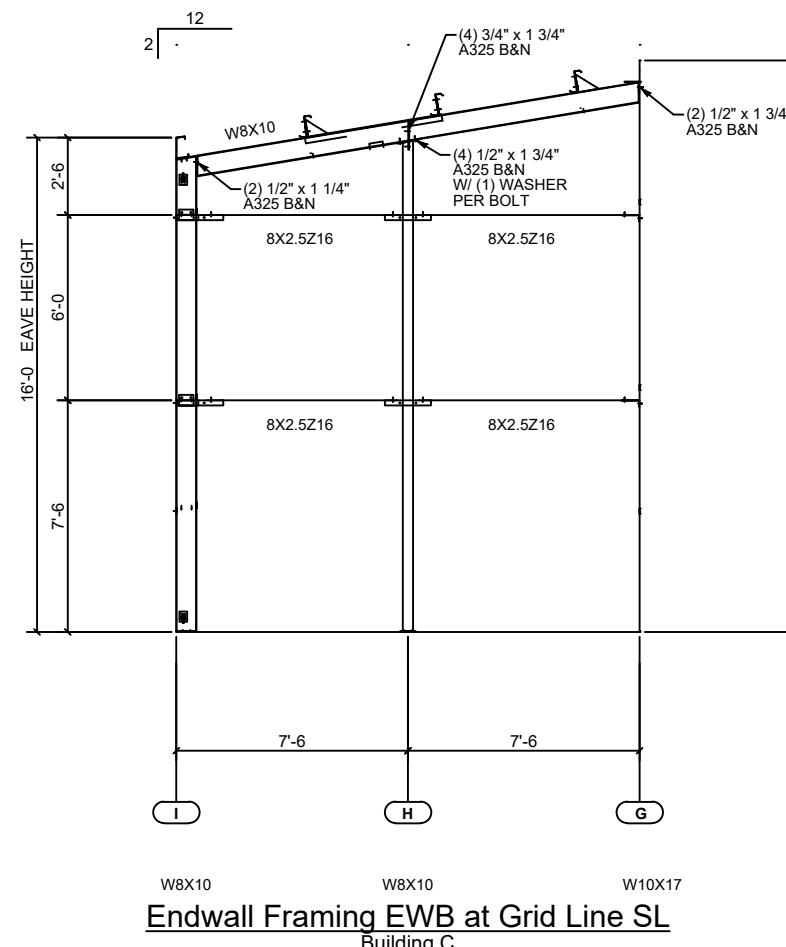
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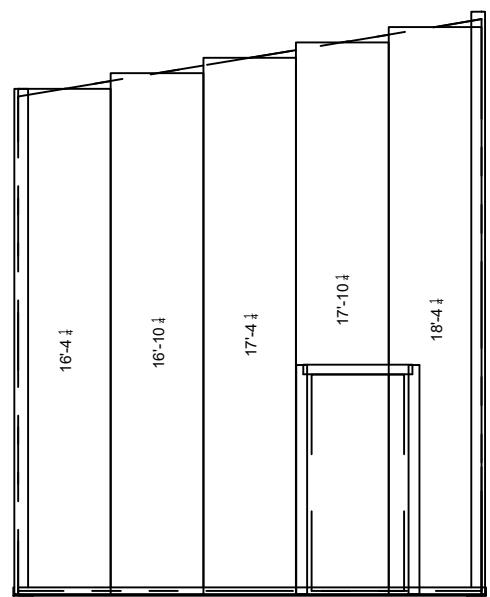
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5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Project Name & Location: IEENE BREWER 5230 CARROLL CANYON RD SITE 300 SAN DIEGO, CA 92121-1761						
Customer: IEENE BREWER 5230 CARROLL CANYON RD FALLBROOK, CA 92028-9587 US						
Drawing Status: <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Construction <input type="checkbox"/> Issued For Permit						



PBR Wall Panels
Panel Coverage = 3'-0"
Panel Color = S200 POLAR WHITE
Panel Pkg. Req'd. = PBS-1
Field Cut Panel and Trim as
required per Construction Details



Endwall Sheeting Building C EWB

Project Name & Location:	
Customer:	Project Name & Location:
EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443	KEENE BREWER 2505 WILT RD FALLBROOK, CA 92028-9587 US
Drawing Status: <input type="checkbox"/> Issued For Approval <input checked="" type="checkbox"/> Issued For Construction <input type="checkbox"/> Issued For Permit	
Scale: NOT TO SCALE	
Drawn by: TJT 10/2/25	
Checked by: CNG 10/3/25	
Project Engineer:	
Job Number: 20-B-65935	
Sheet Number: E11 of 17	
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<p>ANAN ALMUGHRABI, P.E. CALIFORNIA P.E. C053882</p>	

ANAN ALMUGHRABI, P.E.
CALIFORNIA P.E. C053882

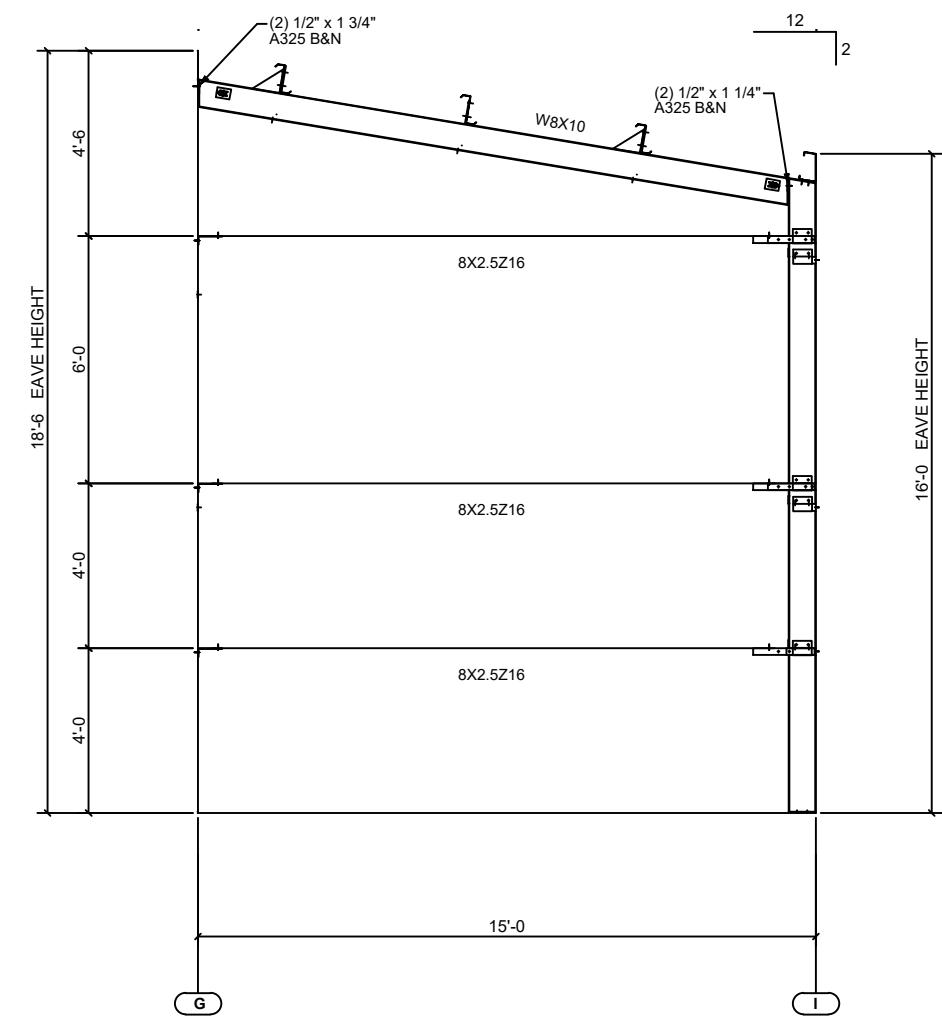
Oct 09, 2025

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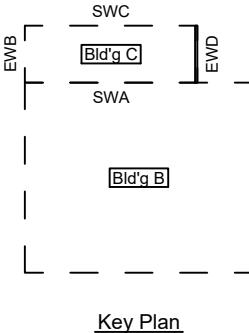
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The diagram illustrates the 'Key Plan' with the following components and their relationships:

- EWB** (Electrical Work Box) is connected to **SWC** (Service Work Center) and **SWA** (Service Work Area).
- SWC** is connected to **Bldg C**.
- SWA** is connected to **Bldg B**.
- EWD** (Electrical Work Drawings) is connected to **Bldg C** and **Bldg B**.

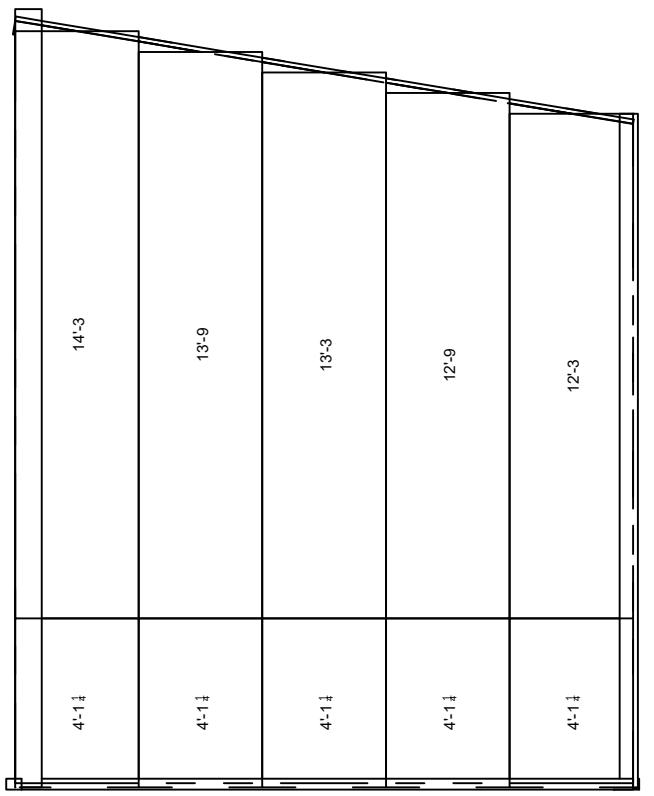


Endwall Framing EWD at Grid Line SL
Building C



PBR Wall Panels
Panel Coverage = 3'-0"
Panel Color = S200 POLAR WHITE
Panel Pkg. Ref'd = PBS-2
Field Cut Panel and Trim as required per Construction Details

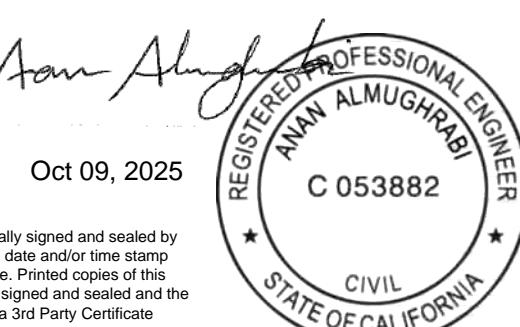
PBR Wainscot Wall Panels
Panel Coverage = 3'-0"
Panel Color = S200 CHARCOAL GRAY
Panel Pkg. Ref'd = PBS-2
Field Cut Panel and Trim as required per Construction Details



Endwall Sheeting Building C EWD

EMPIRE STEEL BUILDINGS		Revision	Date	Description	By	Cr'd
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Project Name & Location: EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD SITE 300 SAN DIEGO, CA 92121-1781						
Customer: KEENE BREWER 2600 MILIT RD FALLBROOK, CA 92028-9587 US						
Drawing Status: <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Construction <input type="checkbox"/> Issued For Permit						
Scale: NOT TO SCALE						
Drawn by: TJT 10/2/25						
Checked by: CNG 10/3/25						
Project Engineer:						
Job Number: 20-B-65935						
Sheet Number: E12 of 17						
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ANAN ALMUGHARABI, P.E. CALIFORNIA P.E. C053882						

Oct 09, 2025

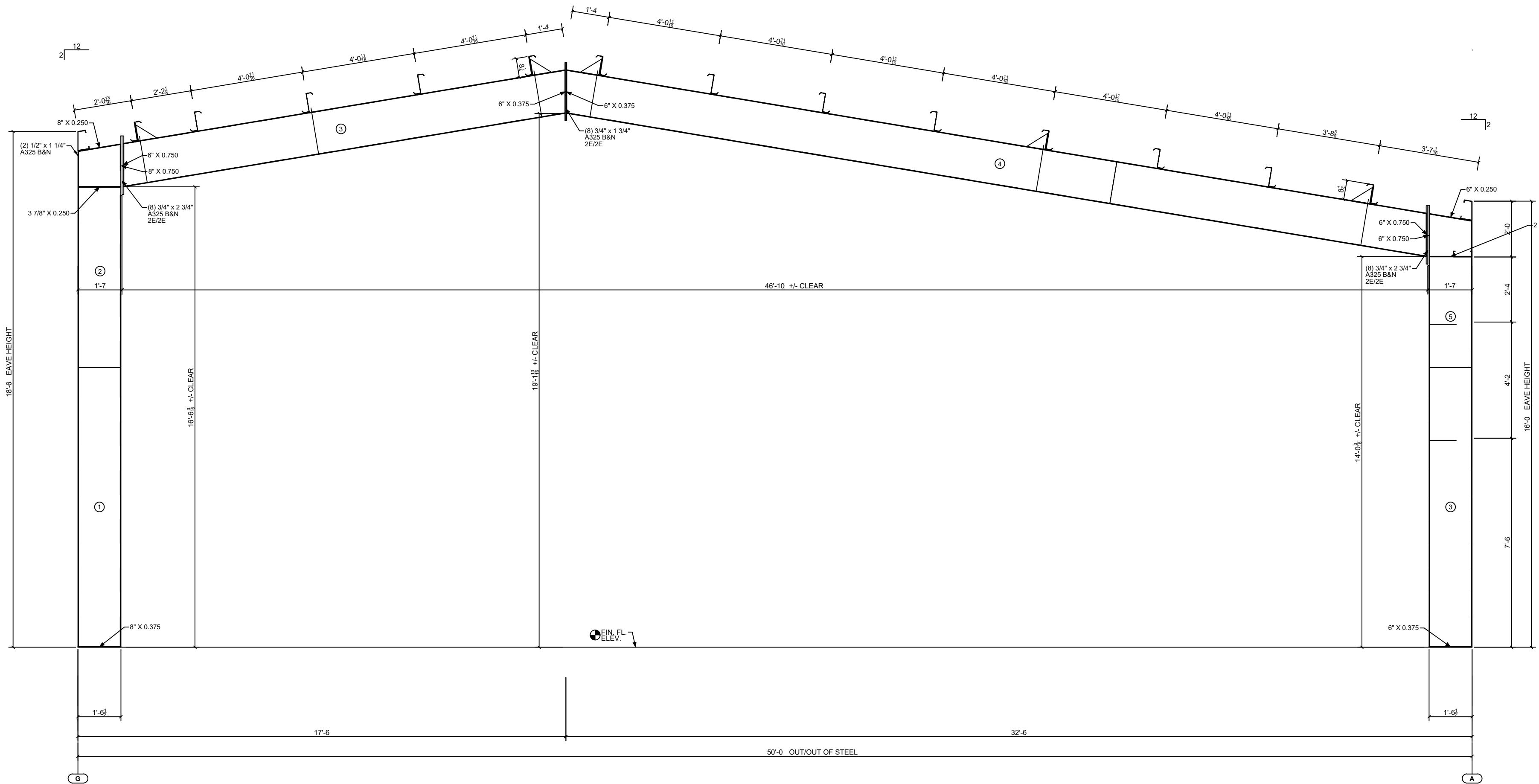


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AND MARKED AS "ISSUED FOR CONSTRUCTION"

APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
R1A	328
R3B	644
CF1A	513
CF3B	352

GENERAL NOTES
FRAME CLEARANCES SHOWN ARE APPROXIMATE AND
MAY VARY DUE TO CONDITIONS (DEFLECTION).
VERTICAL CLEARANCE DIMENSIONS ARE FROM
FINISHED FLOOR REFERENCE ELEVATION.



EMPIRE STEEL BUILDINGS	
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121	
(800) 905-3443	
Customer:	Project Name & Location:
EMPIRE STEEL BUILDINGS	KEENE BREWER 5230 CARROLL CANYON RD SITE 300 SAN DIEGO, CA 92121-1781
Scale:	NOT TO SCALE
Drawn by:	TJT 10/2/25
Checked by:	CNC 10/3/25
Project Engineer:	
Job Number:	20-B-65935
Sheet Number:	E13 of 17
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ANAN ALMUGHARABI, P.E. CALIFORNIA P.E. C053882	

Oct 09, 2025

Anan Almugharabi
REGISTERED PROFESSIONAL ENGINEER
ANAN ALMUGHARABI
C 053882
CIVIL
STATE OF CALIFORNIA

PRIMARY BUILT-UP MEMBER SIZES							
MARK	OUTSIDE FLG THICK	OUTSIDE FLG WIDTH	INSIDE FLG THICK	INSIDE FLG WIDTH	WEB THICK	WEB START DEPTH	WEB END DEPTH
①	0.2500	8"	0.2500	8"	0.1340	18.0000	18.0000
②	0.2500	8"	0.2500	8"	0.2500	18.0000	18.0000
③	0.2500	5"	0.2500	5"	0.1340	18.0000	18.0000
④	0.2500	6"	0.2500	6"	0.1340	18.0000	18.0000
⑤	0.2500	5"	0.2500	5"	0.2500	18.0000	18.0000

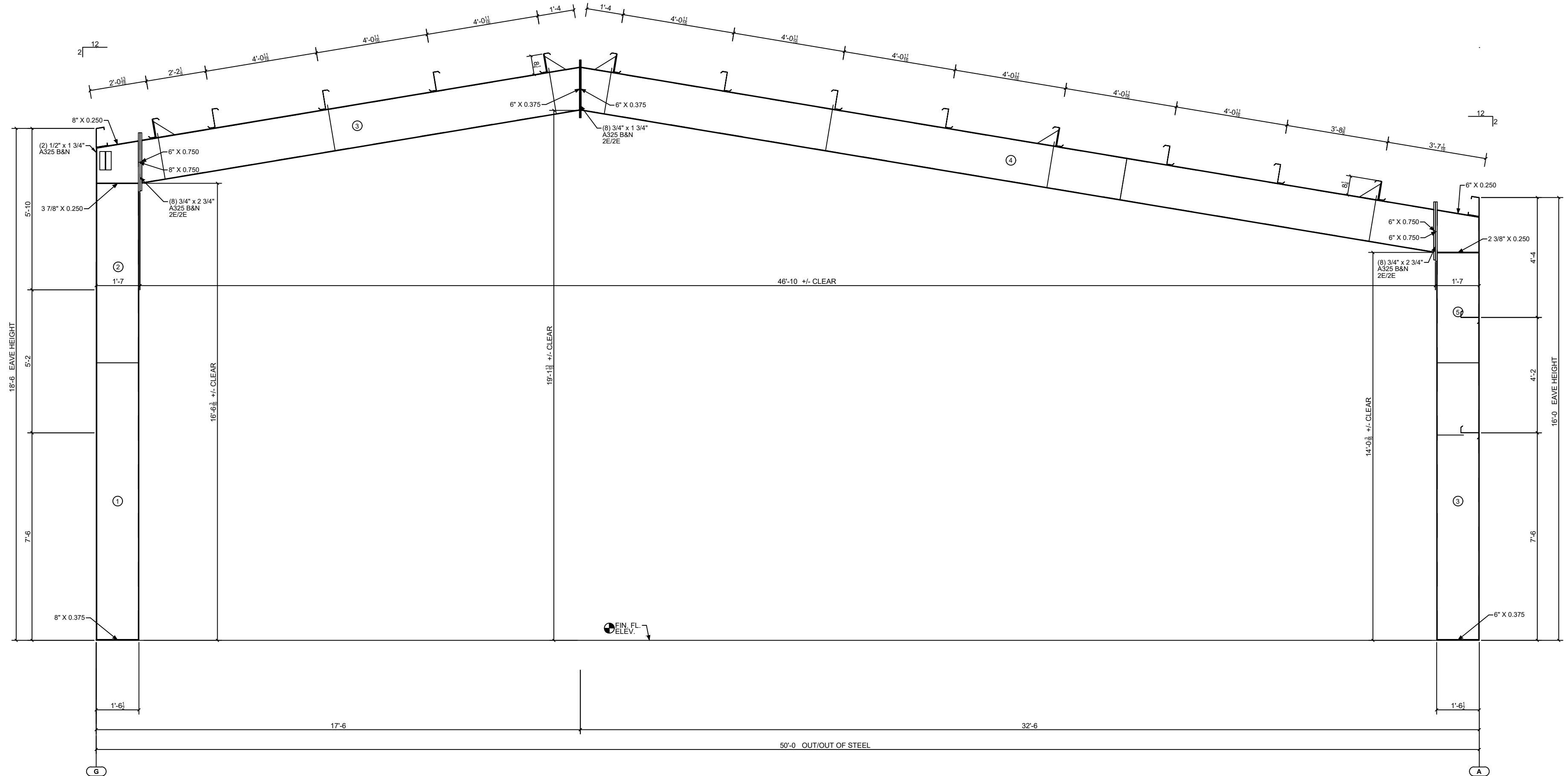
Cross Section at Frame Line 2
Building B

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APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
R2C	328
R4B	644
CF2C	551
CF4B	355

GENERAL NOTES
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EMPIRE STEEL BUILDINGS		Revision	Date	Description	By	Cr/k
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121 (800) 905-3443						
Customer: EMPIRE STEEL BUILDINGS 5230 CARROLL CANYON RD SITE 300 SAN DIEGO, CA 92121-1761		Project Name & Location: KEENE BREWER 2600 MILIT RD FALLBROOK, CA 92028-9587 US				
Drawing Status: <input type="checkbox"/> Issued For Approval <input type="checkbox"/> Issued For Construction <input checked="" type="checkbox"/> Issued For Permit						
Scale:	NOT TO SCALE					
Drawn by:	TJT 10/2/25					
Checked by:	CNC 10/3/25					
Project Engineer:						
Job Number:	20-B-65935					
Sheet Number:	E14 of 17					
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ANAN ALMUGHARABI, P.E. CALIFORNIA P.E. C053882						

Oct 09, 2025

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PRIMARY BUILT-UP MEMBER SIZES

MARK	OUTSIDE FLG THICK		INSIDE FLG THICK		WEB THICK	START DEPTH	END DEPTH
	WIDTH	THICK	WIDTH	THICK			
①	0.2500	8"	0.2500	8"	0.1850	18.0000	18.0000
②	0.2500	8"	0.2500	8"	0.2500	18.0000	18.0000
③	0.2500	5"	0.2500	5"	0.1340	18.0000	18.0000
④	0.2500	6"	0.2500	6"	0.1340	18.0000	18.0000
⑤	0.2500	5"	0.2500	5"	0.2500	18.0000	18.0000

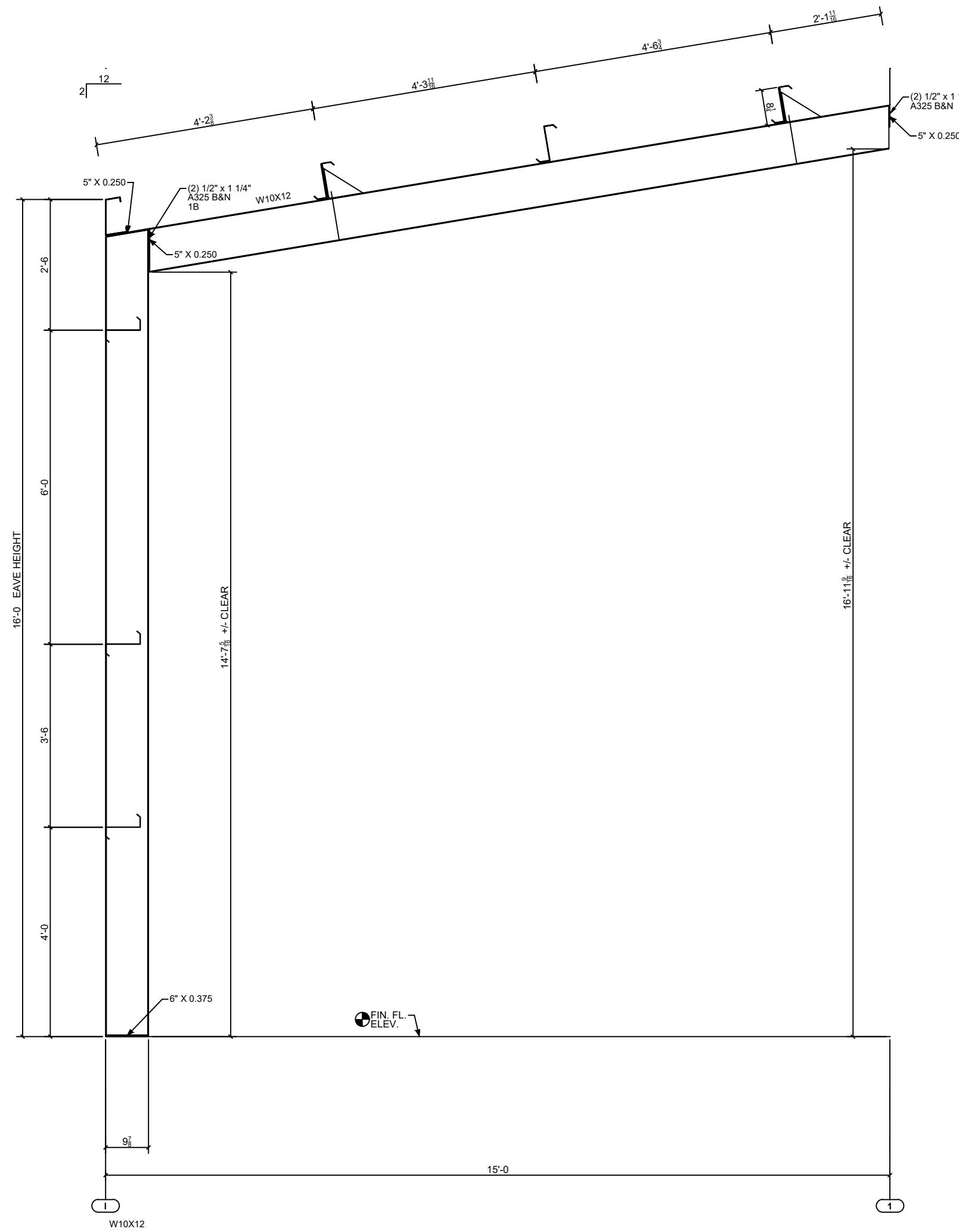
Cross Section at Frame Line 3

Building B

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GENERAL NOTES
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APPROXIMATE MEMBER WEIGHTS		
PART MARK	WEIGHT	By
RY1E	185	Ck
CZ1E	208	



Cross Section at Frame Line 2
Building C

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EMPIRE STEEL BUILDINGS
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121
(800) 905-3443

Project Name & Location:
KEENE BREWER
5230 CARROLL CANYON RD SITE 300
SAN DIEGO, CA 92121-1761

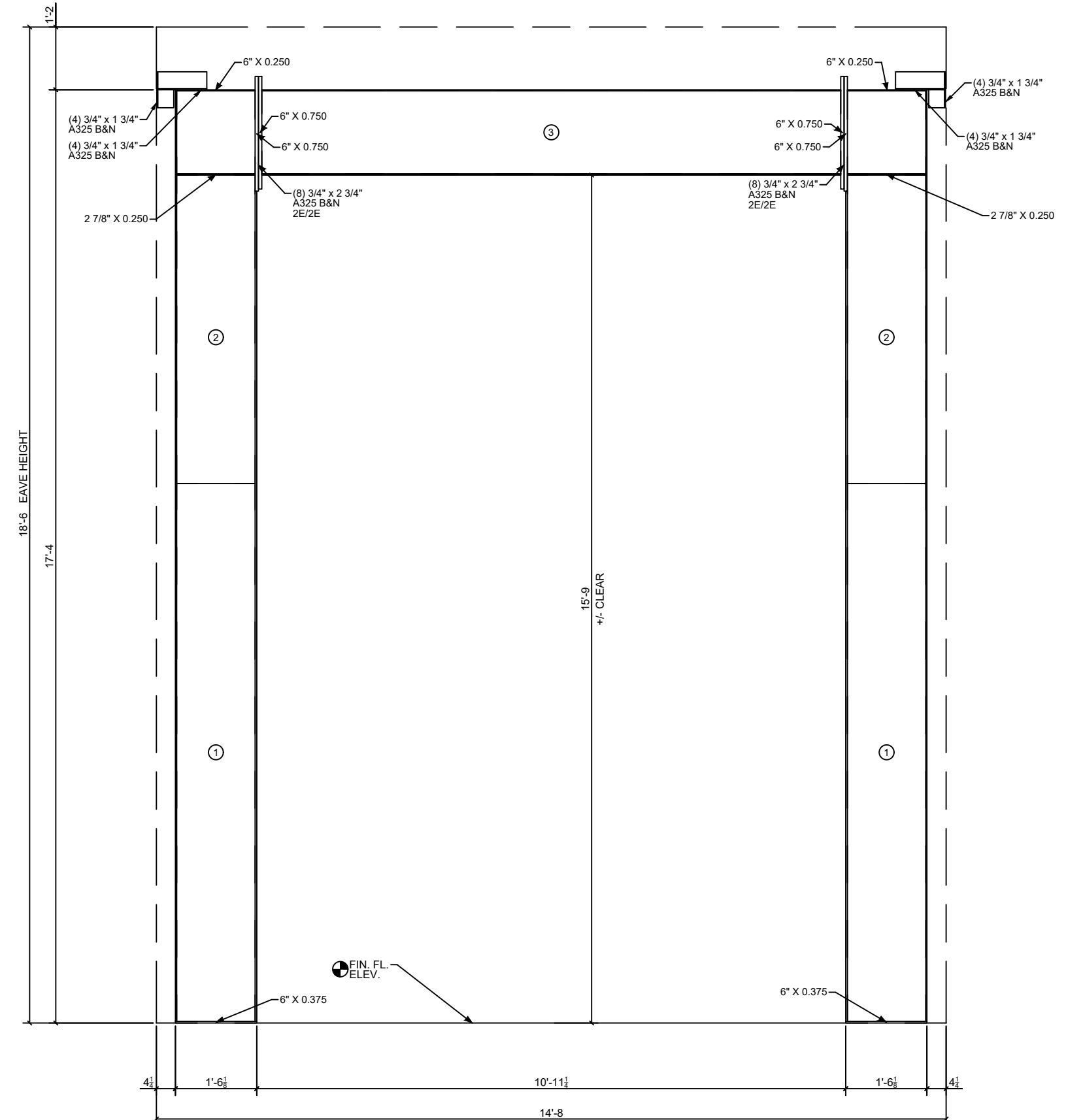
Customer:
NOT TO SCALE
Drawn by: TJT 10/2/25
Checked by: CNG 10/3/25
Project Engineer:
Job Number: 20-B-65935
Sheet Number: E15 of 17

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ANAN ALMUGHRABI, P.E.
CALIFORNIA P.E. C053882

GENERAL NOTES
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APPROXIMATE MEMBER WEIGHTS	
PART MARK	WEIGHT
RKW1F	260
CKW1F	488
CKW2F	484



Portal Frame Cross Section SWC at Grid Line G

Building B

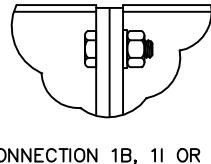
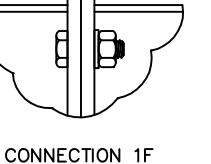
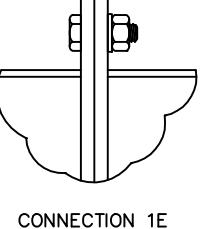
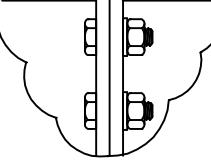
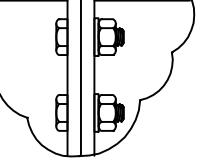
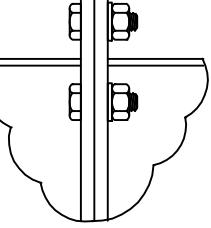
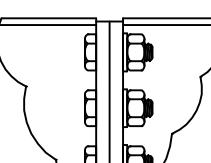
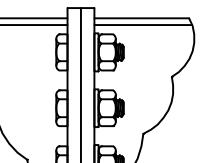
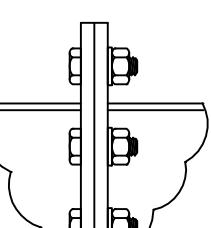
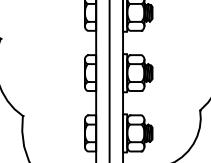
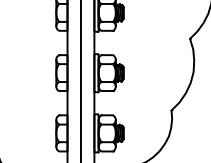
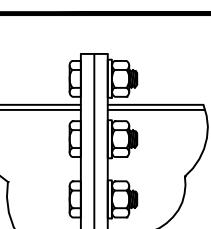
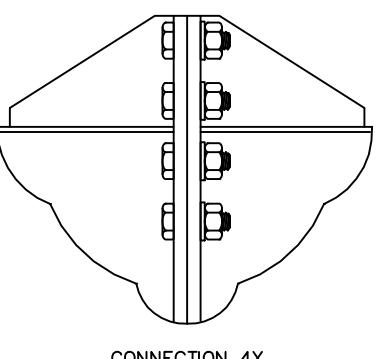
Oct 09, 2025

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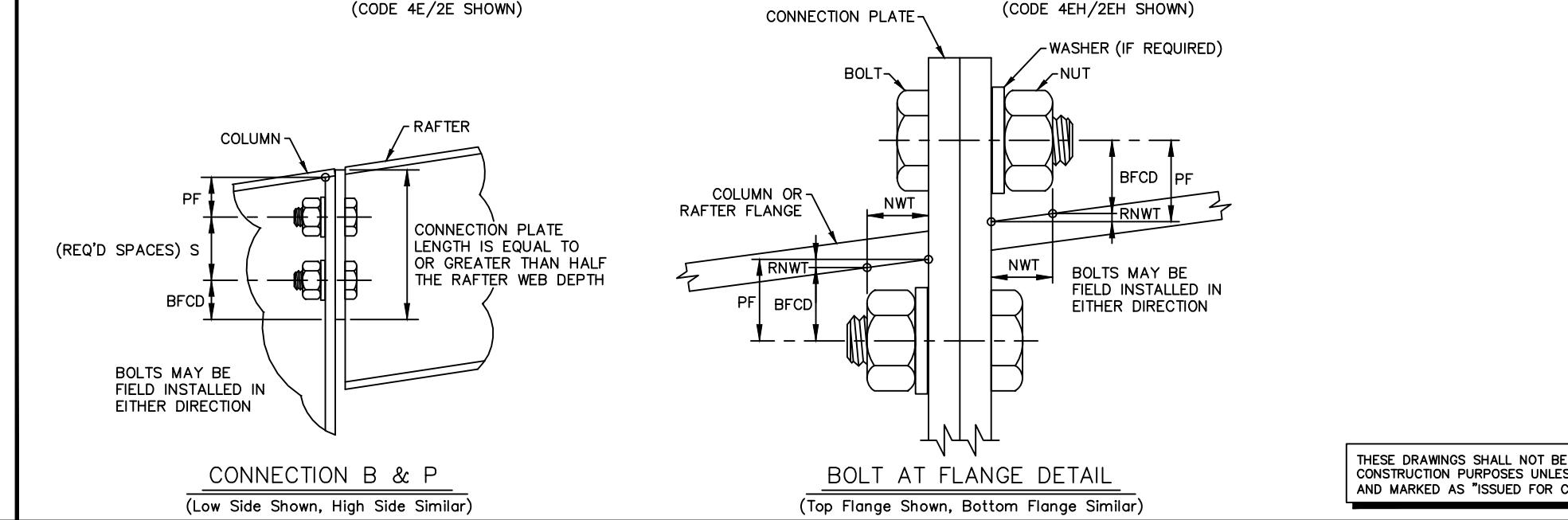
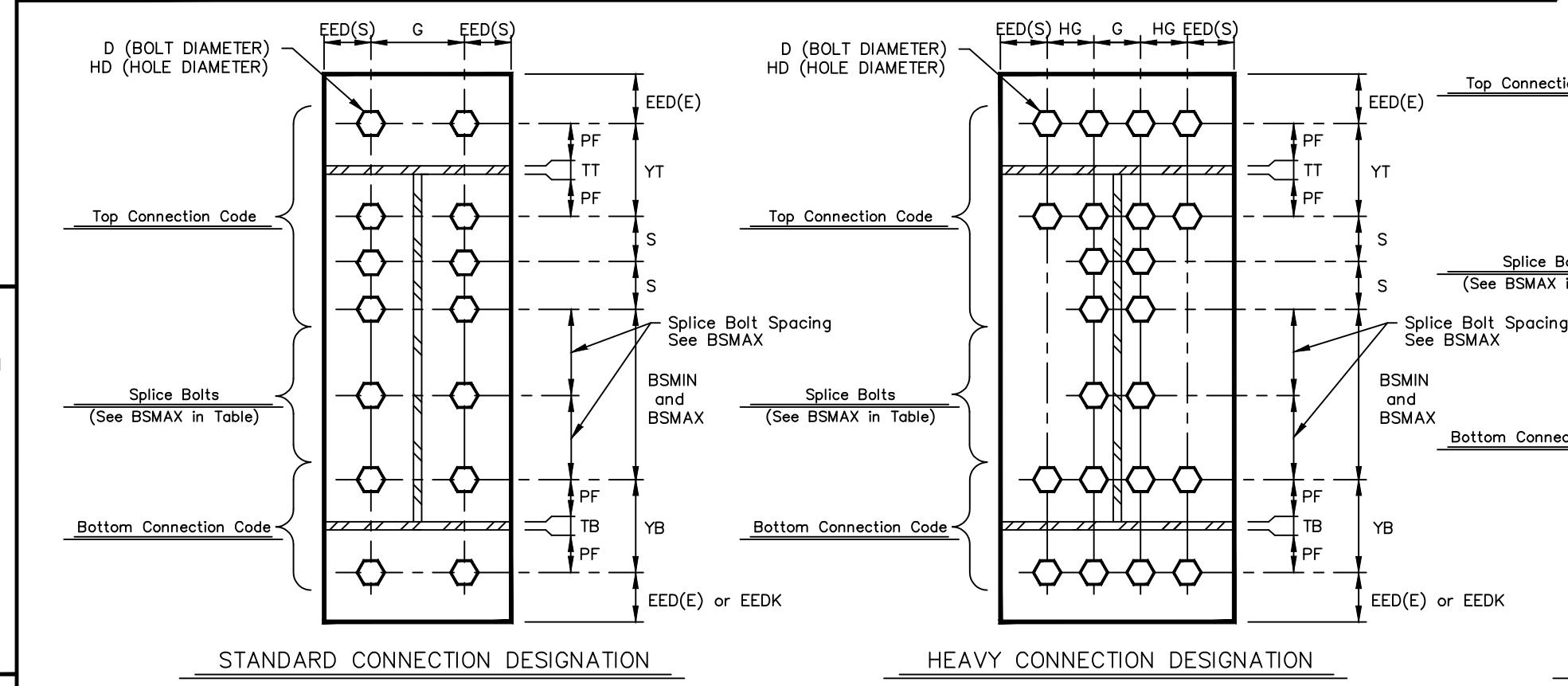
1	0.2500	6"	0.3750	6"	0.1850	17.5000	17.5000
2	0.2500	6"	0.3750	6"	0.2500	17.5000	17.5000
3	0.2500	5"	0.2500	5"	0.1560	18.5000	18.5000

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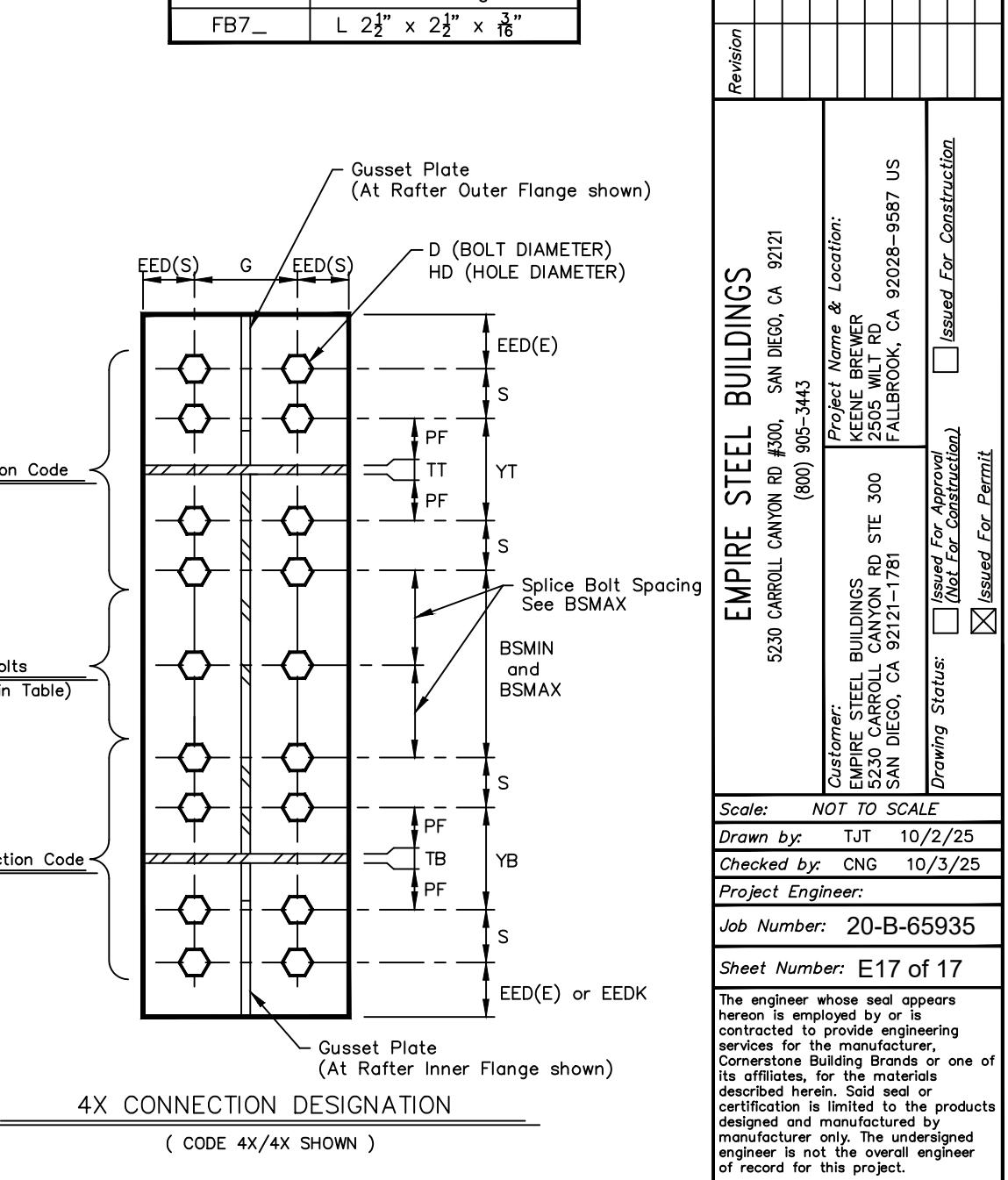


CONNECTION CODES (FOR TOP AND BOTTOM BOLT PATTERN)		
		
<u>CONNECTION 1B, 1I OR 1P</u>	<u>CONNECTION 1F</u>	<u>CONNECTION 1E</u>
		
<u>CONNECTION 2B, 2I OR 2P</u>	<u>CONNECTION 2F</u>	<u>CONNECTION 2E</u>
		
<u>CONNECTION 3B, 3I OR 3P</u>	<u>CONNECTION 3F</u>	<u>CONNECTION 3E</u>
		
<u>CONNECTION 4B, 4I OR 4P</u>	<u>CONNECTION 4F</u>	<u>CONNECTION 4E</u>
	<p>4 E /2 E H CONNECTION DESIGNATION BLANK = STANDARD CONNECTION H = HEAVY CONNECTION BOTTOM CONNECTION CODE BOTTOM QUANTITY OF BOLT ROWS CONNECTION DESIGNATION BLANK = STANDARD CONNECTION H = HEAVY CONNECTION TOP CONNECTION CODE TOP QUANTITY OF BOLT ROWS <u>CONNECTION CODE FORMAT</u></p>	
<u>CONNECTION CODE DESCRIPTION</u>		
<p>B = THIS DESCRIPTION CODE IS USED TO DEFINE SHEAR CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP FLANGE. CONNECTION PLATE LENGTH MUST BE A MINIMUM OF HALF THE RAFTER WEB DEPTH AND SHALL NOT EXCEED THE RAFTER TOTAL DEPTH.</p> <p>E = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED WITH ONE SET OUTSIDE THE TOP OR BOTTOM FLANGE AND THE REMAINING SETS ARE LOCATED INSIDE THE TOP OR BOTTOM FLANGE.</p> <p>F = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP OR BOTTOM FLANGE AND CONNECTION PLATE PROJECTS 1/2" BEYOND THE TOP OR BOTTOM FLANGE.</p> <p>I = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP OR BOTTOM FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP OR BOTTOM FLANGE.</p> <p>P = THIS DESCRIPTION CODE IS USED TO DEFINE SHEAR CONNECTIONS. BOLTS ARE LOCATED INSIDE THE TOP FLANGE AND CONNECTION PLATE IS RECESSED 1/8" BELOW THE TOP FLANGE. CONNECTION PLATE LENGTH MUST BE A MINIMUM OF HALF THE RAFTER WEB DEPTH AND SHALL NOT EXCEED THE RAFTER TOTAL DEPTH.</p> <p>4X = THIS DESCRIPTION CODE IS USED TO DEFINE MOMENT CONNECTIONS. BOLTS ARE LOCATED WITH TWO SETS EACH SIDE OF THE TOP OR BOTTOM FLANGE WITH A GUSSET PLATE OUTSIDE THE TOP AND BOTTOM FLANGE OR COLUMN CAP PLATE.</p>		

NAME	DESCRIPTION FOR A325 BOLT DIMENSIONS						
	1/2"	3/4"	7/8"	1"	1 1/4"	1 1/2"	
D	DIAMETER OF THE BOLT	9/16"	13/16"	15/16"	1 1/16"	1 5/16"	
HD	BOLT HOLE DIAMETER	2 1/2"	3"	4"	3 1/2"	4"	
G	BOLT GAUGE	1"	1 1/8"	1 7/8"	1 1/4"	1 3/8"	
	MAX. WEB THICKNESS (Max. $\frac{5}{16}$ " Fillet Weld) WITHOUT WASHER	3/4"	7/8"	1 5/8"	7/8"	1 7/8"	
HG	HEAVY CONN. BOLT GAUGE	N/A	2 1/4"	2 5/8"	3"	3 3/4"	
S	NORMAL BOLT SPACING	2 1/2"	3"	3 1/4"	3 1/2"	4"	
BSMIN	MINIMUM SPACING BETWEEN TOP & BOTTOM SETS OF BOLTS	1 1/2"	2 1/4"	2 5/8"	3"	3 3/4"	
BSMAX	MAXIMUM BOLT SPACING BETWEEN TOP AND BOTTOM SETS OF BOLTS ON CONNECTION PLATES	2'-0"	2'-0"	2'-0"	2'-0"	2'-0"	
	SPLICE BOLT SPACING	(1/2 BSMAX ($\pm \frac{1}{16}$ ")) WHEN BSMAX = 2'-0 $\frac{1}{16}$ " TO 4'-0 (NOT TO EXCEED 2'-0) (1/3 BSMAX ($\pm \frac{1}{16}$ ")) WHEN BSMAX = 4'-0 $\frac{1}{16}$ " TO 6'-0 (1/4 BSMAX ($\pm \frac{1}{16}$ ")) WHEN BSMAX = 6'-0 $\frac{1}{16}$ " TO 8'-0					
BFCD	MINIMUM BOLT-TO-FLANGE CLEARANCE AT OUT OF NUT SEE BOLT AT FLANGE DETAIL	1 1/2"	1 3/4"	1 7/8"	2 1/4"	2 1/2"	
PF	MINIMUM BOLT-TO-FLANGE CLEARANCE AT CONNECTION PLATE SEE BOLT AT FLANGE DETAIL	(BFCD + RNWT) PF INSIDE OF FLANGE IS INCREASED BASED ON THE YT & YB VALUE. PF FOR CONNECTION B, F, I AND P ARE THE SAME AS USED ON CONNECTION E					
NWT	NUT AND WASHER THICKNESS	SEE BOLT AT FLANGE DETAIL. NUT THICKNESS IS EQUAL TO THE BOLT DIAMETER AND .15625" WASHER THICKNESS IS USED EVEN IF A WASHER IS NOT REQUIRED.					
RNWT	RISE ON NUT AND WASHER THICKNESS						
TT	THICKNESS TOP FLANGE	REFER TO FRAME CROSS SECTION DRAWING FOR LARGEST FLANGE THICKNESS EITHER SIDE OF THE CONNECTION.					
TB	THICKNESS BOTTOM FLANGE						
YT	BOLT SPACING TOP (ROUND UP TO NEXT 1/2", MIN = S)	3" + TT	3 1/2" + TT	3 3/4" + TT	4 1/2" + TT	5" + TT	
YB	BOLT SPACING BOTTOM (ROUND UP TO NEXT 1/2", MIN = S)	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	or TB Sloped	
EED(E)	MINIMUM END EDGE DIMENSION	1 1/4"	1 1/4"	1 1/2"	1 3/4"	2 1/4"	
EED(S)	MINIMUM SIDE EDGE DIMENSION	3/4"	1"	1 1/8"	1 1/4"	1 5/8"	
EEDK	END EDGE DIMENSION AT KNEE CONNECTION	1 3/8"	1 3/8"	1 5/8"	1 7/8"	2 3/8"	
BCWM	MINIMUM BOLT CLEARANCE FROM A FLANGE OR WEB WELD	7/16"	5/8"	3/4"	13/16"	1"	
	WITHOUT WASHER	9/16"	3/4"	7/8"	1"	1 1/4"	
WCSM	MINIMUM WIDTH OF CONNECTION PLATE (Standard Connection)	5"	6"	8"	8"	10"	
WCHM	MINIMUM WIDTH OF CONNECTION PLATE (Heavy Connection)	N/A	10"	12"	12"	16"	
TCMIN	MINIMUM THICKNESS OF CONNECTION PLATE	1/4"	3/8"	7/16"	1/2"	5/8"	
						1"	



Frame Documentation		Page 05-12-10
A325 Connection Bolt Details		Date Mar '24
		Rev. 06
By	Ok'd	
B 4E/2EH	Connection Code (See "Connection Code Format" on this drawing)	
	Connection Location	
CROSS SECTION CONNECTION CODE KEY		(AS SHOWN AT CONNECTIONS ON FRAME CROSS SECTION DRAWINGS)
Flange Brace Material Schedule		
Part Mark	Material	
FB4_	L 2" x 2" x 14 Ga.	
FB5_	L 2" x 2" x 14 Ga.	
FB6_	L 2" x 2" x $\frac{1}{8}$ "	
FB7_	L 2 $\frac{1}{2}$ " x 2 $\frac{1}{2}$ " x $\frac{3}{16}$ "	



Project Name & Location:
EMPIRE STEEL BUILDINGS
5230 CARROLL CANYON RD #300, SAN DIEGO, CA 92121
(800) 905-3443
Issued For Construction:

Customer:
EMPIRE STEEL BUILDINGS
5230 CARROLL CANYON RD SITE 300
FALLBROOK, CA 92028-9587 US
Drawing Status:

Scale: NOT TO SCALE
Drawn by: TJT 10/2/25
Checked by: CNG 10/3/25
Project Engineer:
Job Number: 20-B-65935
Sheet Number: E17 of 17
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ANAN ALMUGHARBI, P.E.
CALIFORNIA P.E. C053882

Oct 09, 2025

REGISTERED PROFESSIONAL ENGINEER
ANAN ALMUGHARBI
C 053882
CIVIL
STATE OF CALIFORNIA

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